

Geotechnical Completion Report

**Stages 1 to 6, Falcons View Subdivision, 153 Lincoln Rolleston Road,
Rolleston**

Issue Date: **28 November 2023**

Miyamoto Ref: **2003576-RP-001[A]**

Prepared for: **Yoursection FV Ltd**



Report Tracking – Stages 1 to 6, Falcons View Subdivision, 153 Lincoln Rolleston Road, Rolleston

Revision	Status	Date	Prepared by	Reviewed by
A	Final	28 November 2023	Joseph Byron-Joyce	Charles McDermott

Authorisation

Author's Signature		Reviewer's Signature	
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1. Introduction and Scope

Miyamoto International NZ Ltd (Miyamoto) has been engaged by Yoursection FV Ltd (the Client) to provide geotechnical engineering services related to the earthworks and building platform preparation for 75 residential lots as part of Stages 1 to 6 of the Falcons View Subdivision at 153 Lincoln Rolleston Road, Rolleston.

Miyamoto have previously provided a 'Geotechnical Report for Proposed Plan Change' (200357-RP-002[A], dated 25 November 2020) for the site.

The purpose of this Geotechnical Completion Report (GCR) is to confirm the suitability of the earthworks and building platforms for building construction. The GCR is also required in order to comply with the Selwyn District Council (SDC) resource consent conditions (refer to Section 3.1 of this Report) and may be used in support of subsequent building consent applications for the individual lots at the SDC.

The following works have been conducted as per the agreed scope of works:

- Construction monitoring during the earthworks:
 - Fill suitability assessment including review of the laboratory testing undertaken for the fill material.
 - Visual inspections and advising the contractor regarding specification compliance.
 - Review of fill compaction verification testing.
- Assessment and reporting included in this Geotechnical Completion Report (GCR).
- Provision of certification documentation including:
 - NZS4404:2010 'Schedule 2A: Statement of professional opinion on suitability of land for building construction'.
 - NZS4431:2022 'Appendix D: Statement of suitability of engineered fill for lightweight structures'.

The subdivision civil design, supervision, and overall management has been completed by Capture Land Development Consultants (Capture), with Ongrade Drainage & Excavation Limited (Ongrade) acting as the main civil contractor completing the civil works.

2. Site Description

The site, legally described as Lot 1 DP 568976 as contained in Record of Title 1024686, is approximately 10 ha in area and is located to the west of Lincoln Rolleston Road, ~3 km south of State Highway 1.

The site is generally flat and prior to development comprised grassed paddocks with shelter belt plantings.

The typical soil profile at the site is shown in Table 1.

Table 1: Typical soil profile

Layer	Typical thickness (m)	Soil Description
Tp	0.3	Topsoil, SILT, brown, with rootlets.
ML	0.3 to 0.8	SILT and Sandy SILT, low plasticity, pale brown, very stiff to hard.
GW	>10.0	Sandy GRAVEL, fine to coarse grained, grey, sub-rounded to rounded.

The site location including the subdivision layout is presented in Figure 1.

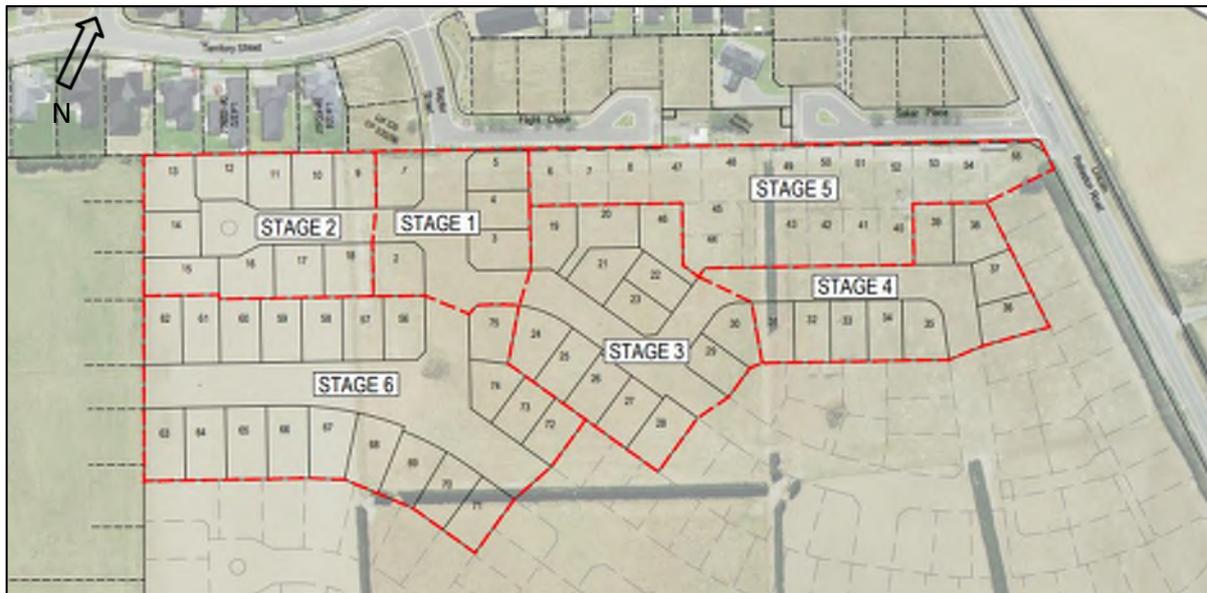


Figure 1: Site location and layout (BASE co, 15/04/23)

3. Earthworks

3.1 SDC Resource Consent Conditions

The resource consent conditions provided by the SDC (approval date 16 February 2023) are detailed within RC225866 and RC225867. This GCR is provided to satisfy conditions 33 and 34 of RC225866 and conditions 3 and 4 of RC225867, as detailed below.

RC225866

Site stability and site works

33. The Consent Holder shall confirm whether any earth fill has been placed on site. All earthworks completed on site are to be carried out in accordance with New Zealand Standard (NZS) 4431:2022 – Engineered fill construction for lightweight structures.

34. At the completion of all earthworks Certificates satisfying the conditions of New Zealand Standard (NZS) 4431:2022 – Engineered fill construction for lightweight structures, are to be provided to the Selwyn District Council. These certificates will detail the extent and nature of all earthworks undertaken.

RC225867

Construction Standards

3. That all earthworks shall be conducted in accordance with the approved engineering plans for subdivision consent 225866 and the Selwyn District Council Engineering Code of Practice.

The Consent Holder shall confirm whether any earth fill has been placed on site. All earthworks completed on site are to be carried out in accordance with New Zealand Standard (NZS) 4431:2022 – Engineered fill construction for lightweight structures.

4. At the completion of all earthworks Certificates satisfying the conditions of New Zealand Standard (NZS) 4431:2022 – Engineered fill construction for lightweight structures, are to be provided to the Selwyn District Council. These certificates will detail the extent and nature of all earthworks undertaken.

3.2 General

The earthworks design for the subdivision was completed by Capture Land Development Consultants (Capture) and was designed to raise / lower the grade to appropriate levels for the residential lots and create appropriate fall for drainage. The earthworks plan and as-built levels are included in Appendix A.

The earthworks were carried out between June and September 2023 by Ongrade Drainage & Excavation Limited (Ongrade), with Miyamoto, Capture, the SDC, and Yoursection FV Ltd completing regular site visits to observe earthworks and civil works at the site.

The initial design completed by Capture included for a total of 3,800 m³ of soil to be cut and placed as engineered fill across the site, with a net balance between cutting and filling resulting in no requirement for importation of fill or removal of soil for the residential lots. Importation of material was required for construction of roading, service trenches, and soakage pits.

Due to thicker topsoil than anticipated in some areas and encountering unsuitable foundation soils in a former 'borrow pit', a relatively small volume of imported fill was required to complete the earthworks for the residential lots.

Earthworks were carried in general accordance with the following New Zealand Standards:

- NZS4431:2022 - Engineered fill construction for lightweight structures.
- NZS4404:2010 - Land Development and Subdivision Infrastructure.

3.3 Cutting and Topsoil Stripping

All residential lots required cutting and / or filling to achieve the desired grade, this necessitated topsoil to be stripped and stockpiled for future spreading. Miyamoto completed visual inspection of topsoil removal during regular site visits and through examination of contractor provided photographs.

Following excavation of topsoil, the majority of the excavated material comprised natural silt and sandy silt which was stockpiled to be used as site won engineered fill in the works (refer to Section 3.4 of this Report).

Cutting to waste (off-site disposal) of soils was completed in one discrete area of a former 'borrow pit' beneath lots 41, 42, 43, 49, 50, and 51, where unsuitable foundation soils (landfill and general refuse) were identified. The unsuitable material was excavated / removed exposing natural sandy

gravel deposits at a depth of greater than 3.5 m below ground level. Miyamoto completed several inspections during and after completion of unsuitable soil removal.

3.4 Filling

The bulk of the engineered fill for the residential lots comprised site won silt and sandy silt sourced from spoil created from the ‘cut’ lots, services and roading alignments. The site won fill material was sampled and tested at a laboratory for Particle Size Distribution (PSD), and determination of the dry density / water content relationship and assessed to be suitable for use as engineered fill (refer to Appendix B for laboratory test certificates).

The site won material had a suitable in-situ moisture content without the requirement for moisture conditioning and was stockpiled and shaped / battered into bunds along road alignments prior to placement as engineered fill. The shaping of the bunds allowed for water shedding to maintain suitable moisture condition of the soil during rainfall events.

Inspection of the stockpiled site won material was completed during regular site visits to confirm it was representative of the material tested in the laboratory. In general, there was very little variability of the material across the site.

The site won fill was placed and tracked in place / compacted with a 60-ton bulldozer, a methodology similar to that of a sheep’s foot roller where the heavy plant imparts a large load directly to the freshly placed material and mechanically penetrates the surface of the fill material with the tracks.

Nuclear Densometer (NDM) testing of the placed and compacted fill material was completed by SGNT limited to verify adequate compaction was achieved. Typically, a minimum of two NDM tests were completed per lot per 250 mm of filling with additional testing completed where fill thickness exceeded 250 mm. The results of the NDM testing are included in Appendix C.

Imported ‘pit run’ material comprising well graded sandy gravel with cobbles (sourced from Wheatsheaf Quarry) was utilised to fill the ‘borrow pit’ at lots 41, 42, 43, 49, 50, and 51. Where used, this material was capped with a minimum of 0.3 m of silt and sandy silt material. The imported material was placed in layers of approximately 250 mm thickness and compacted with a vibratory drum roller. NDM testing was completed at approximately 1 m vertical intervals and to the final lift (refer to Appendix C).

3.5 Earthworks Summary

A summary of the cut / fill earthworks and distribution across the site are shown in Table 2 and the as-built levels are provided in Appendix A.

Table 2: Earthworks summary

Stage	Lots within Stage	Fill Lots	Fill Type	Cut Lots
1	1,2,3,4,5	1,2,3	Site won - Silt	4, 5
2	9, 10, 11, 12, 13, 14, 15, 16, 17, 18	9, 10, 11, 12, 13, 14, 15, 16, 17, 18	Site won - Silt	-
3	19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30	21, 22, 23, 24, 25, 26, 27, 28, 29, 30	Site won - Silt	19, 20
4	31, 32, 33, 34, 35, 36, 37, 38, 39	31, 32, 33, 34, 35, 36, 37, 38, 39	Site won - Silt	-

Stage	Lots within Stage	Fill Lots	Fill Type	Cut Lots
5	6, 7, 8, 40, 41, 42, 43, 44, 45, 47, 48, 49, 50, 51, 52, 53, 54, 54, 55	40, 41, 42, 47, 48, 49, 50, 51, 52	Site won – Silt (all fill lots within stage) Imported – Pit run (lots 41, 42, 43, 49, 50, and 51)	6, 7, 8, 44, 45, 53, 54, 55
6	56, 57, 58, 59, 60, 61, 62, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 73, 74, 75	60, 61, 62, 72, 73, 74, 75	Site won – Silt	56, 57, 58, 59, 63, 64, 65, 66, 67, 68, 69, 70, 71

3.6 Certification

Based on site observations, laboratory testing and in-situ testing of placed and compacted material, in combination with earthworks planning, design, and construction being completed by experienced developers, engineers and contactors, it is our professional opinion that the earthworks for the residential lots have been completed to a suitable standard for residential development.

The following two documents are appended to this GCR (refer Appendix D and E):

- NZS4404:2010 - ‘Schedule 2A: Statement of professional opinion on suitability of land for building construction’.
- NZS4431:2022 - ‘Appendix D: Statement of suitability of engineered fill for lightweight structures’.

4. Building Development Design Considerations

4.1 Foundation Suitability

All residential lots within Stages 1 to 6 of the subdivision are considered to have a low liquefaction vulnerability with future seismic performance expected to be equivalent to MBIE Technical Category (TC) 1 as per the MBIE Guidance (2012).

As such, and with consideration of the assessment herein, foundations in accordance with MBIE (2012) TC1 are considered suitable for NZS3604:2011 compliant buildings, notably this includes NZS3604:2011 foundations and ‘waffle slab’ foundations.

4.2 Bearing Capacity and Other Considerations

The ground conditions across the site within the residential lots will typically comprise topsoil of varying thickness (0.3 to 0.4 m typically) overlying engineered and natural silt and sandy silt soils. In some locations sandy gravel may be encountered beneath topsoil.

Site specific testing should be completed to verify the available Geotechnical Ultimate Bearing Capacity (GUBC) of the soils underlying topsoil.

Preliminarily, assuming conventional residential foundations comprising ‘NZS 3604 type’ slab on grade with thickened edge beams, shallow timber piles of 0.45 m diameter, or waffle slab foundations with 0.3 m wide footings, foundation designers may assume 200 kPa GUBC beneath topsoil within the SILT and Sandy SILT soils at a minimum of 0.4 mbgl, however, this will need to be verified prior to construction.

Foundation designers should consider the effects of the interface between filled and non-filled ground on the performance of the structure.

4.3 Additional Considerations

A geotechnical plan review of proposed residential developments and the foundation design is recommended and considered best practice to ensure the recommendations of this report have been taken into consideration.

Construction monitoring of foundation excavations is recommended to ensure ground conditions encountered are as expected.

5. Limitations

This report is subject to the following limitations:

- This report has been prepared by Miyamoto for the Client for the purpose/s agreed with the Client (Purpose). Miyamoto accepts no responsibility for the validity, appropriateness, sufficiency or consequences of the Client using the report for purposes other than for the Purpose.
- This report is not intended for general publication or circulation. This report is not to be reproduced by the Client except in relation to the Purpose, without Miyamoto's prior written permission. Miyamoto disclaims all risk and all responsibility to any third party.
- This report is provided based on the various assumptions contained in the report.
- Miyamoto's professional services are performed using a degree of care and skill reasonably exercised by reputable consultants providing the same or similar services as at the date of this report.
- The Client is responsible for ensuring that the design of any foundations ensures the functionality of the building under SLS level loads.
- The sub surface information has been obtained from investigation carried out at discrete locations, which by their nature only provide information about a relatively small volume of subsoils. While Miyamoto has taken reasonable skill and care in carrying out the investigation to determine the subsoil condition, the subsoil condition could differ substantially from the results of any sampling investigation. Miyamoto is not responsible for and does not accept any liability in respect of any difference between the actual subsoil conditions and the results of our investigation.
- Where the Client provides information to Miyamoto, including design calculations and drawings of the as-built structure, or where the report indicates that we have obtained and/or relied upon information provided from a third party, Miyamoto has not made any independent verification of this information except as expressly stated in the report. Miyamoto assumes no responsibility for any inaccuracies in, or omissions to, that information.
- A change in circumstances, facts, information after the report has been provided may affect the adequacy or accuracy of the report. Miyamoto is not responsible for the adequacy or accuracy of the report as a result of any such changes.

References

Ministry of Business, Innovation, and Employment, 2012. *Repairing and rebuilding houses affected by the Canterbury earthquakes.*

New Zealand Standard NZS3604:2011. Timber-framed buildings.

New Zealand Standard NZS4404:2010. Land Development and Subdivision Infrastructure.

New Zealand Standard NZS4431:2022. Engineered fill construction for lightweight structures.

Appendix A: Earthworks Plan and As-built Levels



This drawing remains the property of Capture Land Limited and may not be reproduced or amended without written permission. No liability shall be accepted for unauthorised use of this drawing.

LEGEND:

- 40.47 — MAJOR CONTOUR (0.5m)
- - - - MINOR CONTOUR (0.1m)
- 40.47 FINISHED LEVEL

NOTES:

1. CONTOURS SHOWN ON THIS PLAN DEPICT THE FINISHED TOPSOIL LEVELS OVER THE SITE.
2. ASBUILT INFORMATION WAS COLLECTED AND SUPPLIED BY ONGRADE DRAINAGE & EXCAVATION LTD AND CENTA SURVEY LTD.

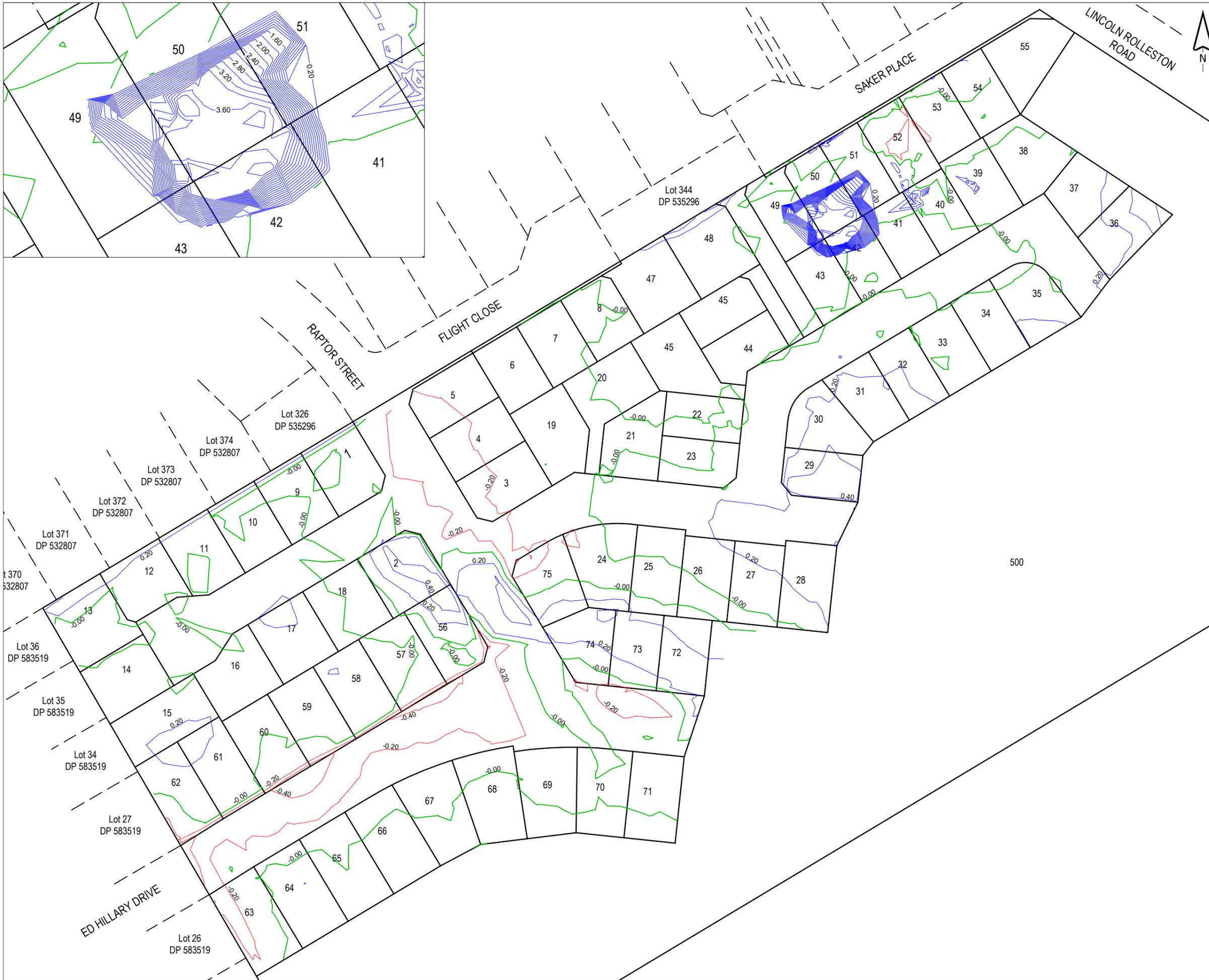
ORIGIN OF LEVELS: BURNHAM NO 2 (1127)
 REDUCED LEVEL: 69.5942m
 DATUM: LVD 1937 (DEC 13)

REV	DATE	REVISION DETAILS	ISSUED
A	27/11/23	FOR INFORMATION	DG



CLIENT		
YOURSECTION FV LTD		
PROJECT		
FALCONS VIEW - STAGE 1-6		
DRAWING TITLE		
ASBUILT PLAN EARTHWORKS FINISHED LEVELS AND CONTOURS		
STATUS	SCALE	SIZE
FOR INFORMATION	1:1250	A3
PROJECT	DRAWING NO	REVISION
1041-1	AB-01	A

c:\users\dangowan\capture\clients - documents\yoursection\1041-1 ab-01-02 earthworks asbuilt plan rev a



This drawing remains the property of Capture Land Limited and may not be reproduced or amended without written permission. No liability shall be accepted for unauthorised use of this drawing.

- LEGEND:**
- 0.20 CUT CONTOUR (0.2m)
 - ZERO CONTOUR
 - 0.20 FILL CONTOUR (0.2m)

- NOTES:**
1. CONTOURS SHOWN ON THIS PLAN DEPICT THE DEPTH OF CUT OR FILL BETWEEN THE ORIGINAL SURFACE STRIPPED OF TOPSOIL AND ANY UNDERCUT AREAS, VERSUS THE FINAL SURFACE PRIOR TO TOPSOIL BEING RESPREAD.
 2. ASBUILT INFORMATION WAS COLLECTED AND SUPPLIED BY ONGRADE DRAINAGE & EXCAVATION LTD AND CENTA SURVEY LTD.

ORIGIN OF LEVELS: BURNHAM NO 2 (1127)
 REDUCED LEVEL: 69.5942m
 DATUM: LVD 1937 (DEC 13)

REV	DATE	REVISION DETAILS	ISSUED
A	27/11/23	FOR INFORMATION	DG



CLIENT
 YOURSECTION FV LTD

PROJECT
 FALCONS VIEW - STAGE 1-6

DRAWING TITLE
 ASBUILT PLAN
 EARTHWORKS CUT/FILL CONTOURS

STATUS FOR INFORMATION
SCALE 1:1250
SIZE A3

PROJECT 1041-1
DRAWING NO AB-02
REVISION A

c:\users\dangowan\captureland\clients - documents\yoursection ltd\1041-falcons view\cad\civilstage 1\asbuilts\1041-1-ab-01-02-earthworks asbuilt plan rev a

Appendix B: Laboratory Test Certificates

Dry Density / Water Content Relationship
New Zealand Standard Compaction Test

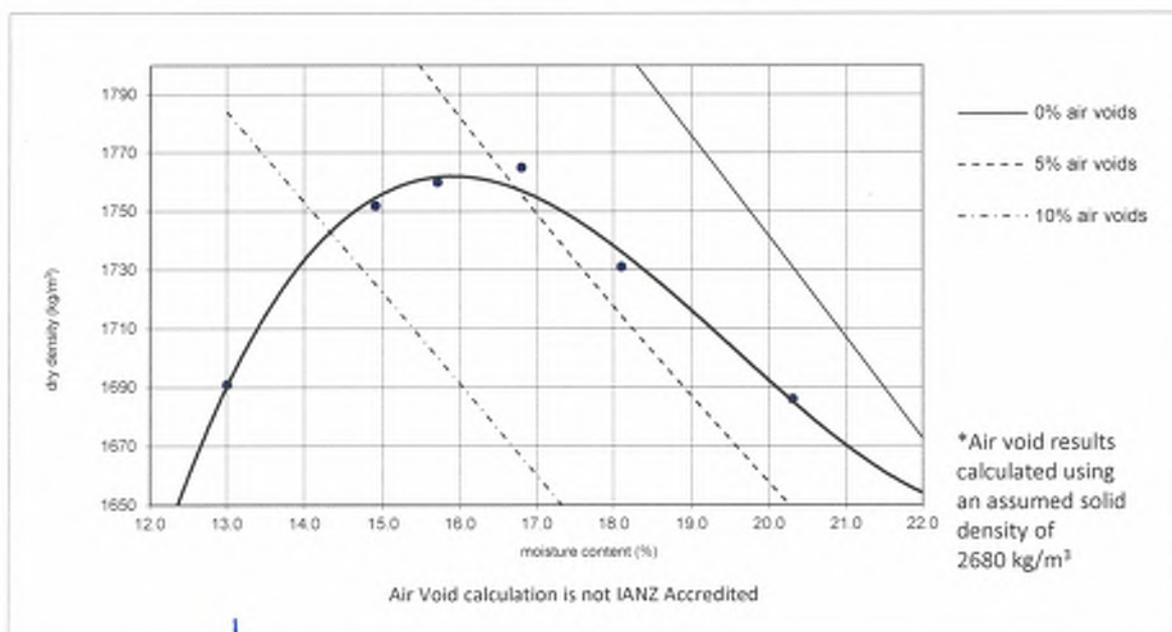
Lab Reference: 0862 / 23

Page 1 of 1 Page

Client: SGNT Limited
Contact Name: Mr S. Gardner
Sample Type: Silt with aggregate
Sample Source: Falcons View, Rolleston (Stage 1 to 6)
Date of Receipt: 8 June 2023
Date of Test: 13 June 2023
Sample Method: Unknown (Sampling method is not IANZ Accredited)
Test Method: NZS 4402:1986 Test 4.1.1 (Standard Compaction)
Results:

Sampled By: S. Gardner
Tested By: J. Tieman

Moisture Content (% by dry mass)	Wet Density (kg/m ³)	Dry Density (kg/m ³)
13.0	1910	1690
14.9	2010	1750
15.7	2040	1760
16.8	2060	1770
18.1	2040	1730
20.3	2030	1690
Maximum Dry Density = 1760 kg/m ³ Optimum Water Content = 16.0 %		
Sample History: Natural. Test performed on sample passing 19.0mm (1.2% of sample removed)		


Date of Issue: 15 June 2023

Approved Signatory: 
 (T. O'Regan, Laboratory Manager)

Checked By: 

This report relates only to the sample tested and may only be reproduced in full.



PITRUN TEST ANALYSIS REPORT

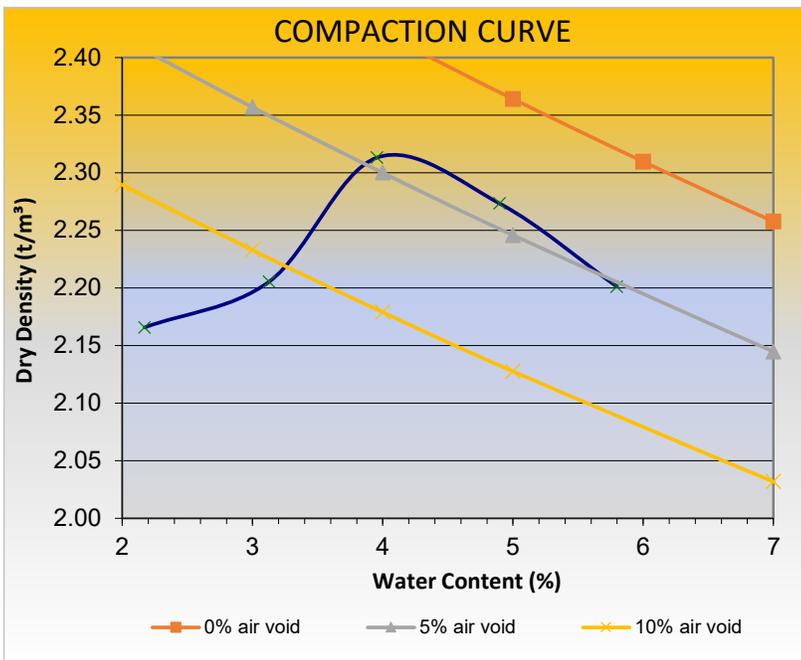
Page 1 of 1

CLIENT:	Winstone Aggregates, PO Box 17 195, Greenlane, Auckland				
CLIENT SAMPLE REF:	Source Property	LABORATORY NO:	P23-0254 - FINAL		
SAMPLE REFERENCE:	Wheatsheaf Quarry	DATE SAMPLED:	19 June 2023		
MATERIAL SOURCE:	Wheatsheaf Quarry	DATE RECEIVED:	19 June 2023		
MATERIAL:	Pitrun	DATE REPORTED:	20 June 2023		

Determination of the Dry Density/Water Content Relationship: NZ Vibrating Hammer Compaction Test - NZS 4402: 1986 Test 4.1.3

Date of Test: 19/06/2023

Test Number:	1	2	3	4	5
Bulk Density (t/m ³)	2.21	2.27	2.40	2.38	2.33
Water Content (%)	2.2	3.1	4.0	4.9	5.8
Dry Density (t/m ³)	2.17	2.21	2.31	2.27	2.20



Maximum Dry Density (t/m³): 2.32

Optimum Water Content (%): 4.0

Test performed on fraction passing (mm): 37.5

Percent retained on the 37.5mm sieve: 28

NOTES:

- 1 Compaction performed on air dried sample
- 2 Solid density of 2.68 t/m³ obtained from this test report P23-0063 to calculate airvoids
- 3 Sampled by G Hayward
- 4 Sample received in damp condition
- 5 Sampling is endorsed by this report

GORDON HAYWARD BSc PGDipSci
LABORATORY MANAGER - KEY TECHNICAL PERSONNEL



All tests reported herein have been performed in accordance with the laboratory's scope of accreditation

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Appendix C: Nuclear Densometer Test Results

Nuclear Density Report

Principal: Mike Niven

Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158

Report No: CHRW04543

Project Name: Falcons View

Testing Details

Site Tested: Fill Lot 1, 9 to 13 Final layer
 Date: 30/08/2023 Time: 12.15
 Material: Silt
 Field methods: NZS 4407:2015 Test 4.2

Compaction Target Details

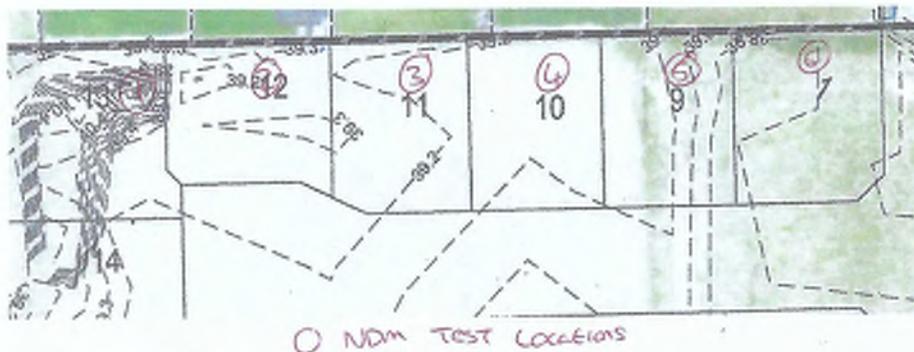
Material Sample ID: External
 Max. Dry Density: 1.76 (t/m³) @ 16.0 %
 Min. Dry Density (t/m³): 1.67
 Solid density: Assumed

Moisture Content Determined by Ndm

Test Results

Site No	Depth (mm)	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	200	15.5	2.17	1.88	107
2	200	15.5	2.03	1.74	100
3	200	16.1	2.12	1.82	104
4	200	19.2	2.10	1.76	100
5	200	12.4	2.16	1.92	109
6	200	11.7	2.15	1.92	109

Ndm test Locations
 not to scale



Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven
 Client: Ongrade Drainage & Excavation Ltd

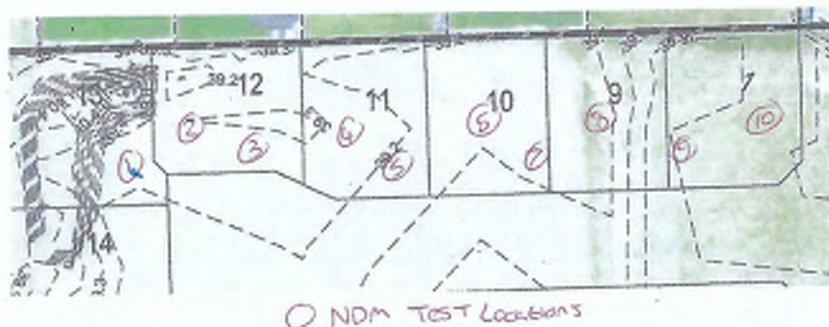
Project No: SGNT02158
 Report No: CHRW04489
 Project Name: Falcons View

Testing Details		Compaction Target Details	
Site Tested: Fill Lot 1, 9 to 13 Final layer		Material Sample ID: External	
Date: 23/08/2023	Time: 16.00	Max. Dry Density: 1.76 (t/m ³) @ 16.0 %	
Material: Silt		Min. Dry Density (t/m ³): 1.67	
Field methods: NZS 4407:2015 Test 4.2		Solid density: Assumed	
Moisture Content Determined by Ndm			

Test Results

Site No	Depth (mm)	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	200	12.2	2.04	1.81	102
2	200	13.9	2.22	1.95	110
3	200	10.3	2.07	1.88	105
4	200	20.8	2.11	1.75	98
5	200	20.1	2.11	1.76	99
6	200	17.4	2.11	1.80	101
7	200	11.6	2.08	1.82	102
8	200	18.0	2.11	1.79	100
9	200	14.4	2.05	1.79	101
10	200	13.5	2.18	1.92	108

Ndm test Locations
 not to scale



Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven

Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158

Report No: CHRIW04559

Project Name: Flacons View

Testing Details

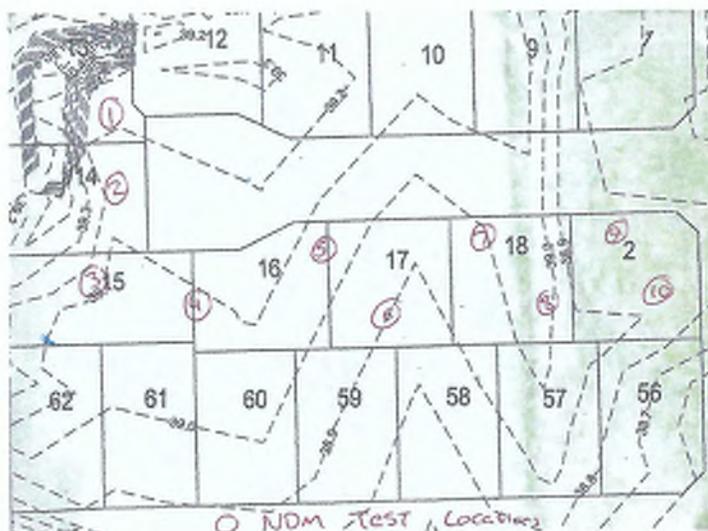
 Site Tested: Fill Lot 2 & 13 to 18 Final layer
 Date: 31/08/2023 Time: 16.00
 Material: Silt
 Field methods: NZS 4407:2015 Test 4.2
 Moisture Content Determined by Ndm

Compaction Target Details

 Material Sample ID: External
 Max. Dry Density: 1.76 (t/m³) @ 16.0 %
 Min. Dry Density (t/m³): 1.67
 Solid density: Assumed

Test Results

Site No	Depth (mm)	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	200	11.8	2.15	1.92	109
2	200	10.3	2.15	1.95	110
3	200	11.9	2.13	1.91	108
4	200	12.6	1.93	1.72	98
5	200	14.8	1.97	1.94	109
6	200	15.6	2.03	1.75	100
7	200	11.6	2.11	1.94	110
8	200	11.9	2.11	1.89	107
9	200	12.6	2.12	1.88	107
10	200	11.8	2.11	1.89	107

 Ndm test Locations
 not to scale


Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven
 Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158
 Report No: CHRIW04035
 Project Name: Flacons View

Testing Details

Site Tested: Fill Lot 2, 14 to 18, 56 to 62 Final layer
 Date: 12/06/2023 Time: 9.30
 Material: Silt
 Field methods: NZS 4407:2015 Test 4.2

Compaction Target Details

Material Sample ID: External
 Max. Dry Density: 1.76 (t/m³) @ 16.0 %
 Min. Dry Density (t/m³): 1.67
 Solid density: Assumed

Moisture Content Determined by Ndm

Test Results

Site No	Depth (mm)	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	200	20.2	2.05	1.71	97
2	200	20.0	2.03	1.69	96
3	200	18.4	2.01	1.69	96
4	200	19.0	2.10	1.76	100
5	200	19.9	2.10	1.74	99
6	200	18.0	2.97	1.78	101
7	200	18.1	2.09	1.77	101
8	200	16.1	2.06	1.77	101
9	200	17.2	2.07	1.77	100
10	200	18.0	2.06	1.75	99
11	200	19.3	2.07	1.73	98
12	200	18.5	2.08	1.75	99
13	200	15.8	2.12	1.83	104
14	200	18.5	2.13	1.80	102
15	200	19.6	2.07	1.73	98
16	200	19.3	2.07	1.74	97
17	200	18.6	2.08	1.75	100
18	200	16.0	2.05	1.77	101
19	200	14.9	2.12	1.84	105
20	200	17.4	2.08	1.77	101
21	200	19.5	2.13	1.78	101
22	200	19.3	2.12	1.77	101
23	200	19.5	2.10	1.76	100
24	200	19.6	2.08	1.74	99
25	200	17.3	2.06	1.75	100
26	200	15.9	2.07	1.79	102

Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven
 Client: Ongrade Drainage & Excavation Ltd

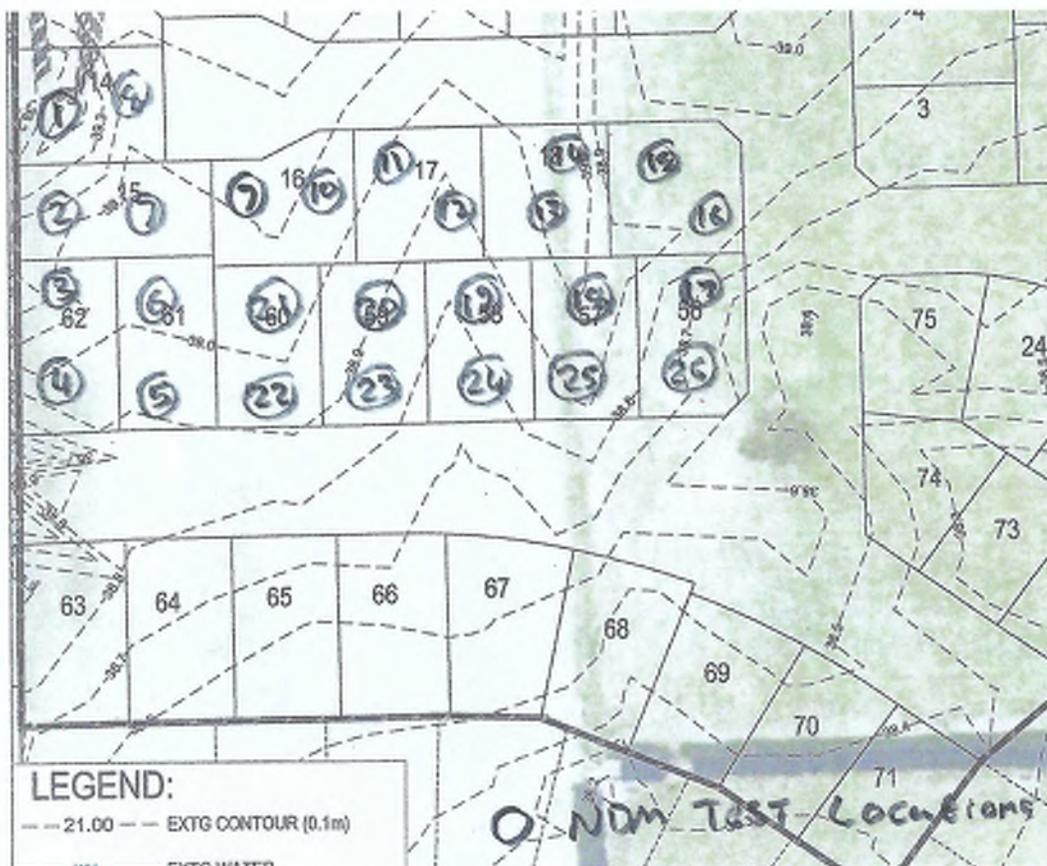
Project No: SGNT02158
 Report No: CHR1W04035
 Project Name: Flacons View

Testing Details

Site Tested: Fill Lot 2, 14 to 18, 56 to 62 Final layer
 Date: 12/08/2023 Time: 9.30
 Material: Silt
 Field methods: NZS 4407:2015 Test 4.2

Compaction Target Details

Material Sample ID: External
 Max. Dry Density: 1.76 (t/m³) @ 16.0 %
 Min. Dry Density (t/m³): 1.67
 Solid density: Assumed



Ndm test Locations
 not to scale

Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven

Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158

Report No: CHRIW04191

Project Name: Flacons View

Testing Details

Site Tested: Fill Lot 21 to 23 Final layer

Date: 30/06/2023

Time: 10.00

Material: Silt

Field methods: NZS 4407:2015 Test 4.2

Moisture Content Determined by Ndm

Compaction Target Details

Material Sample ID: External

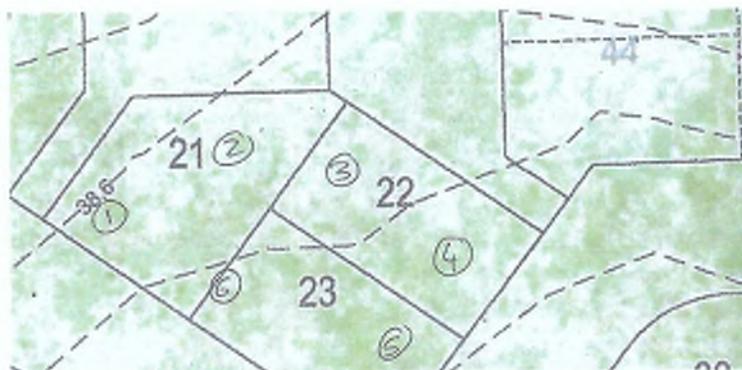
Max. Dry Density: 1.76 (t/m³) @ 16.0 %

Min. Dry Density (t/m³): 1.67

Solid density: Assumed

Test Results

Site No	Depth (mm)	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	200	13.8	2.10	1.85	105
2	200	14.2	2.00	1.75	100
3	200	12.8	1.99	1.76	100
4	200	12.7	1.98	1.75	100
5	200	13.1	1.97	1.74	99
6	200	14.1	2.01	1.76	100



○ NDM TEST LOCATIONS

Ndm test Locations
 not to scale

Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven
 Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158
 Report No: CHR1W04246
 Project Name: Flacons View

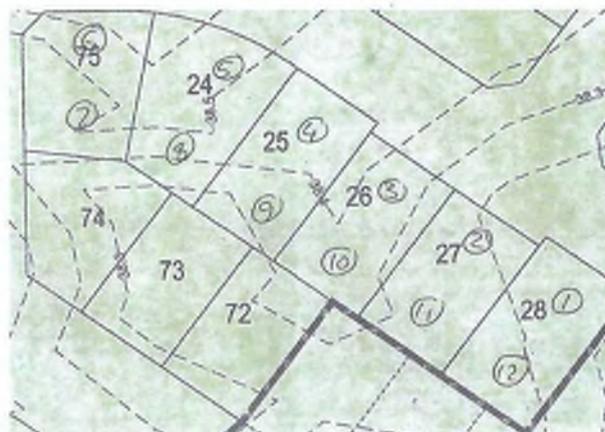
Testing Details		Compaction Target Details	
Site Tested:	Fill Lot 24 to 28 & 75 Final layer	Material Sample ID:	External
Date:	18/07/2023 Time: 17.00	Max. Dry Density:	1.76 (t/m ³) @ 16.0 %
Material:	Silt	Min. Dry Density (t/m ³)	1.67
Field methods:	NZS 4407:2015 Test 4.2	Solid density:	Assumed

Moisture Content Determined by Ndm

Test Results

Site No	Depth (mm)	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	200	17.3	2.03	1.73	98
2	200	18.7	2.04	1.72	98
3	200	15.6	2.02	1.75	99
4	200	17.3	1.98	1.69	96
5	200	16.6	1.99	1.70	97
6	200	17.4	2.02	1.72	98
7	200	17.7	2.03	1.73	98
8	200	16.6	2.20	1.68	95
9	200	12.8	2.13	1.89	107
10	200	18.4	1.99	1.68	95
11	200	15.2	2.05	1.78	101
12	200	15.4	2.01	1.74	99

Ndm test Locations
 not to scale



○ NDM Test Locations

Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven
 Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158
 Report No: CHRIW04060
 Project Name: Flacons View

Testing Details

Site Tested: Fill Lot 24 to 28, 72 to 75 Final layer
 Date: 14/06/2023 Time: 16.20
 Material: Silt
 Field methods: NZS 4407:2015 Test 4.2

Compaction Target Details

Material Sample ID: External
 Max. Dry Density: 1.76 (t/m³) @ 16.0 %
 Min. Dry Density (t/m³): 1.67
 Solid density: Assumed

Moisture Content Determined by Ndm

Test Results

Site No	Depth (mm)	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	200	19.0	2.14	1.80	102
2	200	17.4	2.02	1.72	98
3	200	17.9	2.06	1.75	100
4	200	14.8	2.06	1.79	102
5	200	14.5	2.11	1.84	105
6	200	15.9	2.07	1.79	102
7	200	17.3	2.06	1.75	100
8	200	15.5	2.06	1.79	101
9	200	14.8	2.12	1.85	105
10	200	13.9	2.05	1.80	102
11	200	20.5	2.06	1.71	97
12	200	18.9	2.10	1.76	100
13	200	19.4	2.05	1.72	98
14	200	19.5	2.13	1.78	101
15	200	19.1	2.08	1.75	99
16	200	18.7	2.09	1.76	100
17	200	18.2	2.09	1.77	101
18	200	18.6	2.09	1.76	100

Ndm test Locations
 not to scale



Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven

Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158

Report No: CHRIW04342

Project Name: Falcons View

Testing Details

Site Tested: Fill Lot 29 to 35 Final layer

Date: 2/08/2023

Time: 7.30

Material: Silt

Field methods: NZS 4407:2015 Test 4.2

Moisture Content Determined by Ndm

Compaction Target Details

Material Sample ID: External

Max. Dry Density: 1.76 (t/m³) @ 16.0 %

Min. Dry Density (t/m³): 1.67

Solid density: Assumed

Test Results

Site No	Depth (mm)	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	200	17.5	2.09	1.78	101
2	200	15.9	2.10	1.81	103
3	200	14.1	2.18	1.91	109
4	200	14.9	2.14	1.86	106
5	200	18.5	2.10	1.77	101
6	200	16.3	1.99	1.71	97
7	200	11.0	2.13	1.87	106
8	200	13.1	2.15	1.70	108
9	200	14.8	2.08	1.81	103
10	200	18.3	2.07	1.75	100
11	200	16.0	2.08	1.78	101
12	200	16.6	2.09	1.79	102
13	200	14.1	2.15	1.88	106
14	200	15.2	2.13	1.85	105

Ndm test Locations
 not to scale



○ NDM Test Locations

Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven

Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158

Report No: CHRIW04382

Project Name: Falcons View

Testing Details

Site Tested: Fill Lot 36 to 38 test 2 First Layer 1,3 to 7 Final layer

Date: 8/08/2023

Time: 16.20

Material: Silt

Field methods: NZS 4407:2015 Test 4.2

Moisture Content Determined by Ndm

Compaction Target Details

Material Sample ID: External

Max. Dry Density: 1.76 (t/m³) @ 16.0 %

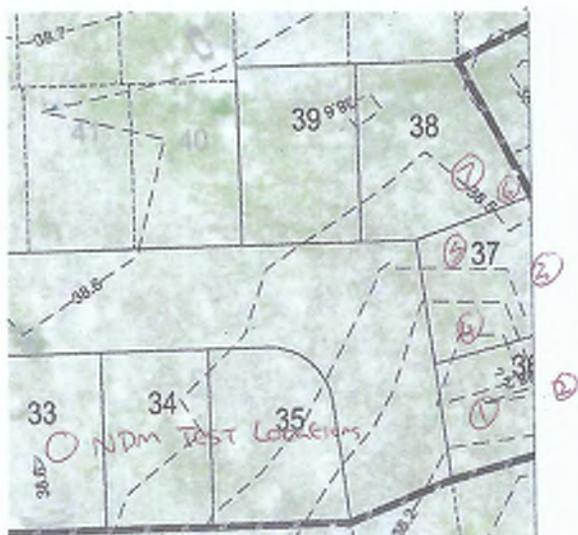
Min. Dry Density (t/m³): 1.67

Solid density: Assumed

Test Results

Site No	Depth (mm)	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	200	11.4	2.11	1.89	108
2	200	13.3	2.20	1.94	110
3	200	21.4	2.05	1.69	96
4	200	11.7	2.15	1.93	109
5	200	12.5	2.15	1.91	108
6	200	12.7	2.04	1.81	103
7	200	11.4	2.09	1.88	107

Ndm test Locations
 not to scale



Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven

Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158

Report No: CHR1W04378

Project Name: Falcons View

Testing Details

Site Tested: Fill Lot 38 to 40 & 52 to 54 Final layer
 Date: 8/08/2023 Time: 10.00
 Material: Silt
 Field methods: NZS 4407:2015 Test 4.2

Compaction Target Details

Material Sample ID: External
 Max. Dry Density: 1.76 (t/m³) @ 16.0 %
 Min. Dry Density (t/m³): 1.67
 Solid density: Assumed

Moisture Content Determined by Ndm

Test Results

Site No	Depth (mm)	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	200	16.3	1.97	1.70	96
2	200	13.6	1.97	1.73	98
3	200	12.1	1.99	1.78	101
4	200	14.0	1.99	1.75	99
5	200	12.7	2.00	1.78	101
6	200	12.8	1.99	1.76	100

Ndm test Locations
 not to scale



Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven

Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158

Report No: CHRIW04144

Project Name: Flacons View

Testing Details

Site Tested: Fill Lot 47 & 48 Final layer

Date: 26/08/2023

Time: 14.30

Material: Silt

Field methods: NZS 4407:2015 Test 4.2

Moisture Content Determined by Ndm

Compaction Target Details

Material Sample ID: External

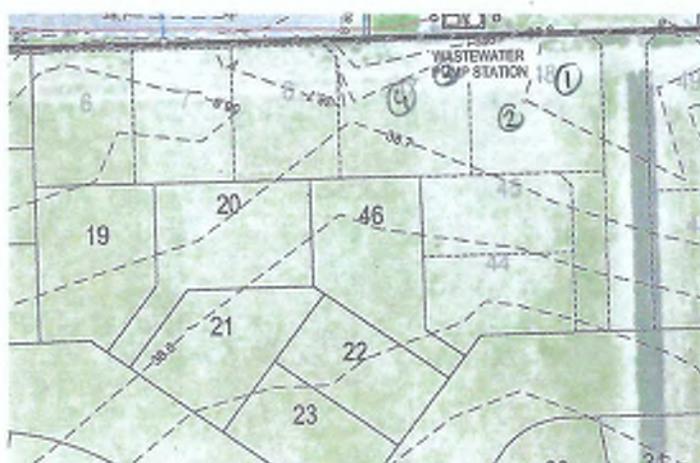
Max. Dry Density: 1.76 (t/m³) @ 16.0 %

Min. Dry Density (t/m³): 1.67

Solid density: Assumed

Test Results

Site No	Depth (mm)	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	200	20.8	2.04	1.69	96
2	200	20.5	2.05	1.70	97
3	200	20.9	2.04	1.69	96
4	200	18.8	2.02	1.70	96



○ NDM Test Locations

Ndm test Locations
not to scale

Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven
 Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158
 Report No: CHR/W04435
 Project Name: Falcons View

Testing Details

Site Tested: Fill Lot 49 to 52 Final layer
 Date: 16/08/2023 Time: 12.34
 Material: Silt
 Field methods: NZS 4407:2015 Test 4.2

Compaction Target Details

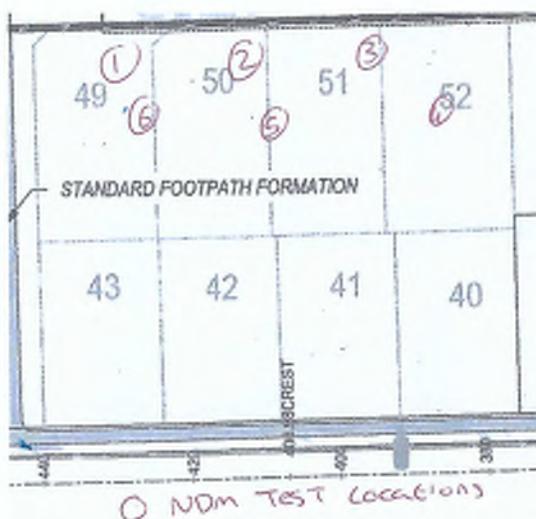
Material Sample ID: External
 Max. Dry Density: 1.76 (t/m³) @ 16.0 %
 Min. Dry Density (t/m³): 1.67
 Solid density: Assumed

Moisture Content Determined by Ndm

Test Results

Site No	Depth (mm)	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	200	17.5	2.09	1.78	101
2	200	15.9	2.10	1.81	103
3	200	14.1	2.18	1.91	109
4	200	14.9	2.14	1.86	106
5	200	18.5	2.10	1.77	101
6	200	16.3	1.99	1.71	97

Ndm test Locations
 not to scale



Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven

Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158

Report No: CHRW04589

Project Name: Flacons View

Testing Details

Site Tested: Fill Lot 56 to 62 Final layer

Date: 5/09/2023

Time: 8.10

Material: Silt

Field methods: NZS 4407:2015 Test 4.2

Moisture Content Determined by Ndm

Compaction Target Details

Material Sample ID: External

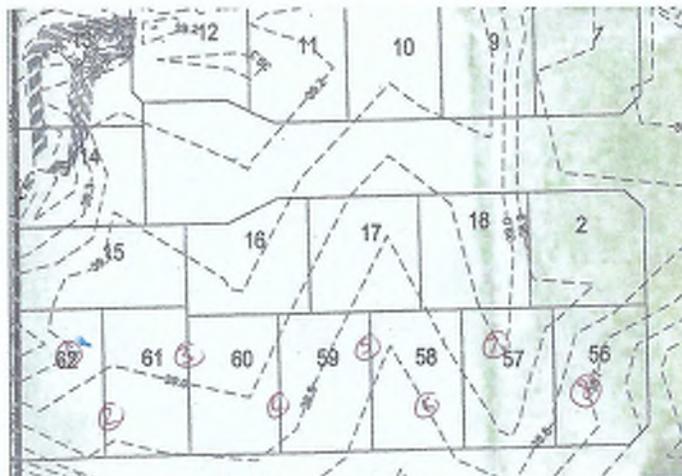
 Max. Dry Density: 1.76 (t/m³) @ 16.0 %

 Min. Dry Density (t/m³): 1.67

Solid density: Assumed

Test Results

Site No	Depth (mm)	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	200	15.9	2.12	1.83	104
2	200	15.4	2.11	1.82	104
3	200	16.1	2.13	1.84	104
4	200	15.2	2.14	1.86	105
5	200	13.9	2.10	1.84	105
6	200	13.2	2.07	1.83	104
7	200	13.4	2.07	1.83	104
8	200	14.5	2.12	1.85	105

 Ndm test Locations
 not to scale


O NDM TEST LOCATIONS

Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven
 Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158
 Report No: CHRW04258
 Project Name: Flacons View

Testing Details

Site Tested: Fill Lot 72 to 74 Final layer
 Date: 19/07/2023 Time: 16.00
 Material: Silt
 Field methods: NZS 4407:2015 Test 4.2

Compaction Target Details

Material Sample ID: External
 Max. Dry Density: 1.76 (t/m^3) @ 16.0 %
 Min. Dry Density (t/m^3): 1.67
 Solid density: Assumed

Moisture Content Determined by Ndm

Test Results

Site No	Depth (mm)	Moisture (%)	Wet Density (t/m^3)	Dry Density (t/m^3)	Relative Compaction (%)
1	200	13.5	2.09	1.84	105
2	200	11.5	1.97	1.72	98
3	200	13.5	1.94	1.71	97
4	200	11.5	2.19	1.92	108
5	200	15.9	2.14	1.85	105
6	200	14.7	1.95	1.70	97

Ndm test Locations
 not to scale



○ NDM TEST LOCATIONS

Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven

Client : Ongrade Drainage & Excavation Ltd

Project No: SGNT02158

Report No: CHRIW04060B

Project Name: Flacons View

Testing Details

Site Tested: Fill Lot 24 to 28 , 72 to 75 First layer
 Date: 14/06/2023 Time: 16.20
 Material : Silt
 Field methods : NZS 4407:2015 Test 4.2

Compaction Target Details

Material Sample ID: External
 Max. Dry Density : 1.76 (t/m³) @ 16.0 %
 Min. Dry Density (t/m³) 1.67
 Solid density: Assumed

Moisture Content Determined by Ndm

Test Results

Site No	Depth (mm)	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	200	19.0	2.14	1.80	102
2	200	17.4	2.02	1.72	98
3	200	17.9	2.06	1.75	100
4	200	14.8	2.06	1.79	102
5	200	14.5	2.11	1.84	105
6	200	15.9	2.07	1.79	102
7	200	17.3	2.06	1.75	100
8	200	15.5	2.06	1.79	101
9	200	14.8	2.12	1.85	105
10	200	13.9	2.05	1.80	102
11	200	20.5	2.06	1.71	97
12	200	18.9	2.10	1.76	100
13	200	19.4	2.05	1.72	98
14	200	19.5	2.13	1.78	101
15	200	19.1	2.08	1.75	99
16	200	18.7	2.09	1.76	100
17	200	18.2	2.09	1.77	101
18	200	18.6	2.09	1.76	100

Ndm test Locations
 not to scale



Comments

MDD Method : Test was conducted externally and is not accredited by this laboratory.
 (This report supersedes chriw4060)

Nuclear Density Report

Principal: Mike Niven

Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158

Report No: CHR/W04528

Project Name: Falcons View

Testing Details

Site Tested: Lot 41,42,50,51 Old rubbish Tip Backfill Fifth Layer

Date: 28/08/2023

Time: 16.32

Material: Pit run

Field methods: NZS 4407:2015 Test 4.3

Moisture Content Determined by Ndm

Compaction Target Details

Material Sample ID: External

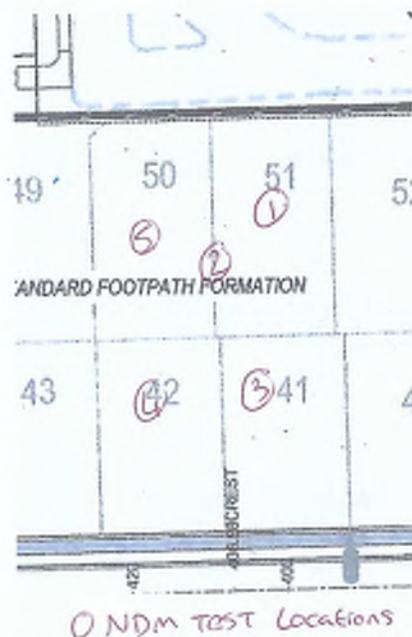
 Max. Dry Density: 2.32 (t/m³) @ 4.0 %

 Min. Dry Density (t/m³): 2.20

Solid density: Assumed

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	6.0	2.50	2.36	102
2	6.5	2.45	2.30	99
3	5.1	2.53	2.39	103
4	5.2	2.45	2.33	101
5	5.3	2.38	2.24	97

 Ndm test locations
 not to scale


Comments

 MDD Method: Test was conducted externally and is not accredited by this laboratory.
 + 200mm on top of Fourth Layer

Nuclear Density Report

Principal: Mike Niven
 Client: Ongrade Drainage & Excavation Ltd

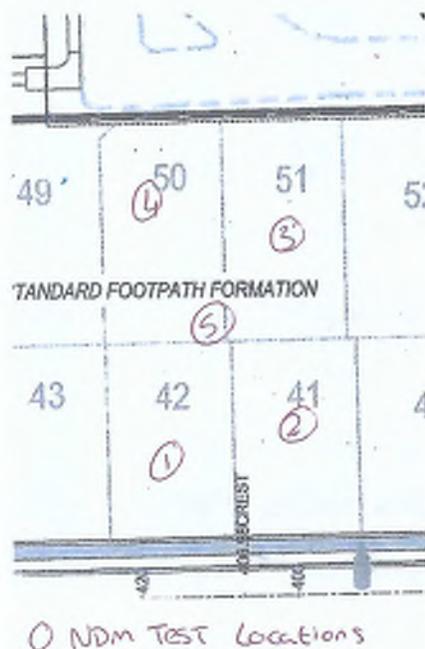
Project No: SGNT02158
 Report No: CHR1W04542
 Project Name: Falcons View

Testing Details		Compaction Target Details	
Site Tested: Lot 41,42,50,51 Old rubbish Tip Backfill Final Layer	Material Sample ID: External		
Date: 30/08/2023	Time: 11.46	Max. Dry Density: 1.76 (t/m ³) @ 16.0 %	
Material: Silt	Min. Dry Density (t/m ³): 1.67		
Field methods: NZS 4407:2015 Test 4.2	Solid density: Assumed		
Moisture Content Determined by Ndm			

Test Results

Site No	Depth (mm)	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	200	17.3	2.09	1.78	101
2	200	18.9	2.09	1.76	100
3	200	13.5	2.18	1.92	109
4	200	15.3	2.04	1.76	101
5	200	13.0	2.05	1.81	103

Ndm test Locations
 not to scale



Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven

Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158

Report No: CHRIW04477

Project Name: Falcons View

Testing Details

Site Tested: Lot 41,42,50,51 Old rubbish Tip Backfill First Layer

Date: 22/08/2023

Time: 12.20

Material: Pit run

Field methods: NZS 4407:2015 Test 4.3

Moisture Content Determined by Ndm

Compaction Target Details

Material Sample ID: External

Max. Dry Density: 2.32 (t/m³) @ 4.0 %

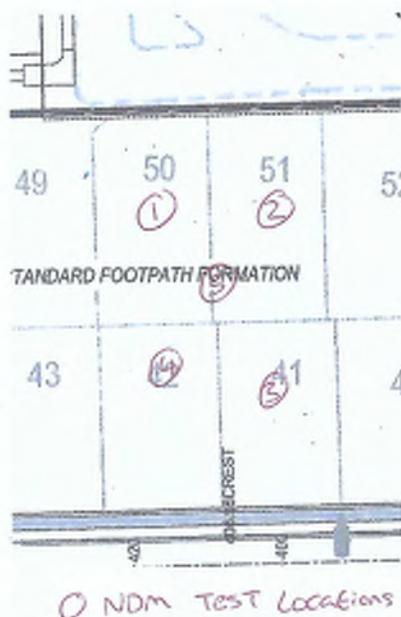
Min. Dry Density (t/m³): 2.20

Solid density: Assumed

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	6.0	2.38	2.24	97
2	5.9	2.50	2.37	102
3	5.9	2.38	2.25	97
4	5.6	2.42	2.29	99
5	4.6	2.50	2.39	103

Ndm test locations
 not to scale



Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven

Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158

Report No: CHRIW04525

Project Name: Falcons View

Testing Details

Site Tested: Lot 41,42,50,51 Old rubbish Tip Backfill Fourth Layer

Date: 28/08/2023

Time: 12.25

Material: Pit run

Field methods: NZS 4407:2015 Test 4.3

Compaction Target Details

Material Sample ID: External

Max. Dry Density: 2.32 (t/m³) @ 4.0 %

Min. Dry Density (t/m³): 2.20

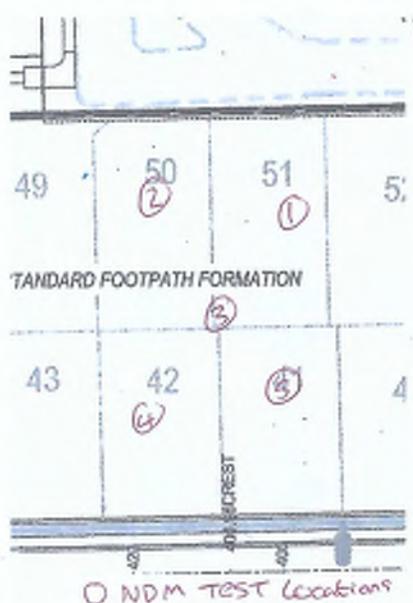
Solid density: Assumed

Moisture Content Determined by Ndm

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	5.8	2.44	2.30	99
2	6.8	2.42	2.26	98
3	5.3	2.37	2.25	97
4	5.0	2.44	2.23	100
5	5.9	2.43	2.30	99

Ndm test locations
 not to scale



Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven

Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158

Report No: CHRIW04483

Project Name: Falcons View

Testing Details

Site Tested: Lot 41,42,50,51 Old rubbish Tip Backfill Second Lay

Date: 23/08/2023

Time: 12.20

Material: Pit run

Field methods: NZS 4407:2015 Test 4.3

Compaction Target Details

Material Sample ID: External

 Max. Dry Density: 2.32 (t/m³) @ 4.0 %

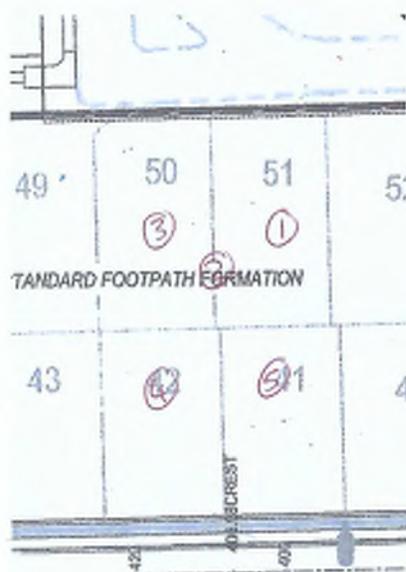
 Min. Dry Density (t/m³): 2.20

Solid density: Assumed

Moisture Content Determined by Ndm

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	5.8	2.38	2.25	97
2	6.3	2.40	2.26	97
3	6.5	2.37	2.22	96
4	5.7	2.48	2.35	101
5	5.5	2.39	2.26	97

 Ndm test locations
 not to scale


O NDM Test Locations

Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Nuclear Density Report

Principal: Mike Niven

Client: Ongrade Drainage & Excavation Ltd

Project No: SGNT02158

Report No: CHR1W04513

Project Name: Falcons View

Testing Details

Site Tested: Lot 41,42,50,51 Old rubbish Tip Backfill Third Layer
 Date: 25/08/2023 Time: 12.00
 Material: Pit run
 Field methods: NZS 4407:2015 Test 4.3

Compaction Target Details

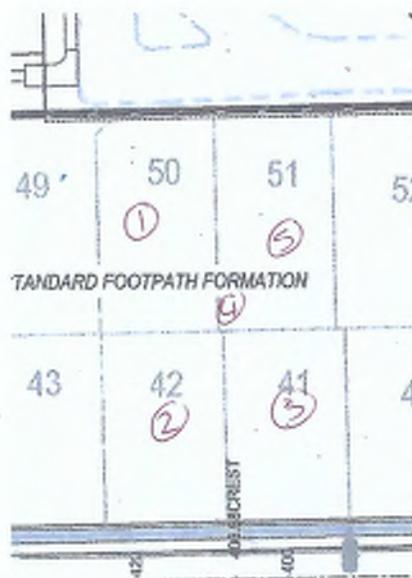
Material Sample ID: External
 Max. Dry Density: 2.32 (t/m³) @ 4.0 %
 Min. Dry Density (t/m³): 2.20
 Solid density: Assumed

Moisture Content Determined by Ndm

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
1	5.7	2.36	2.23	96
2	5.6	2.38	2.25	97
3	5.7	2.41	2.28	98
4	5.0	2.48	2.36	102
5	5.7	2.39	2.26	97

Ndm test locations
 not to scale



Comments

MDD Method: Test was conducted externally and is not accredited by this laboratory.

Appendix D: NZS4404:2010 – Schedule 2A

SCHEDULE 2A
STATEMENT OF PROFESSIONAL OPINION ON SUITABILITY OF LAND FOR BUILDING CONSTRUCTION

Development Falcons View Subdivision.....

Developer Yoursection FV Ltd.....

Location 153 Lincoln Rolleston road, Rolleston.....

I Charles McDermott..... of Miyamoto International NZ Ltd (236 Hereford Street, Christchurch 8011).....
(Full name) (Name and address of firm)

Hereby confirm that:

1. I am a geo-professional as defined in clause 1.2.2 of NZS 4404:2010 and was retained by the developer as the geo-professional on the above development.
2. The extent of my preliminary investigations are described in my Report(s) number 200357-RP-002[A], dated 25 November 2020 and the conclusions and recommendations of that/those document(s) have been re-evaluated in the preparation of this report. The extent of ~~my~~ ^{Miyamoto's} inspections during construction, and the results of all tests and/or re-evaluations carried out are as described in my geotechnical completion report dated 28 November 2023
3. In my professional opinion, not to be construed as a guarantee, I consider that (delete as appropriate):
 - (a) The earth fills shown on the ~~attached~~ ^{in Appendix A of this GCR (Miyamoto 2003576-RP-001[A])} Plan No. have been placed in compliance with the requirements of the Selwyn District Council and ~~my~~ ^{NZS4431:2022} specification.
 - (b) The completed works take into account land slope and foundation stability considerations, subject to the appended foundation recommendations and earthworks restrictions, (which should be read in conjunction with the appended final site contour plan).
 - (c) Subject to 3(a) and 3(b) of this Schedule, the original ground not affected by filling is suitable for the erection of buildings designed according to NZS 3604 provided that:
 - (i) the recommendations included in the Miyamoto GCR (2003576-RP-001[A], dated 28 November 2023) are followed.
 - (ii)
 - (d) Subject to 3(a) and 3(b) of this Schedule, the filled ground is suitable for the erection of buildings designed according to NZS 3604 provided that:
 - (i) the recommendations included in the Miyamoto GCR (2003576-RP-001[A], dated 28 November 2023) are followed.
 - (ii)
 - (e) The original ground not affected by filling and the filled ground are not subject to erosion, subsidence, or slippage in accordance with the provisions of section 106 of the Resource Management Act 1991 provided that:
 - (i) the recommendations included in the Miyamoto GCR (2003576-RP-001[A], dated 28 November 2023) are followed.
 - (ii)

NOTE – These subclauses may be deleted or added to as appropriate, to include such considerations as expansive soils where excluded from NZS 3604, and site seismic characteristics as covered in clause 3.1.3 of NZS 1170.5.

4. This professional opinion is furnished to the TA and the developer for their purposes alone on the express condition that it will not be relied upon by any other person and does not remove the necessity for the normal inspection of foundation conditions at the time of erection of any building.
5. This certificate shall be read in conjunction with my geotechnical report referred to in clause 2 above and shall not be copied or reproduced except in conjunction with the full geotechnical completion report.

Signed 

Date 28 November 2023

Charles McDermott
BEng(Hons) CMEngNZ CPEng IntPE(NZ)

.....
.....
(Name, title, and professional qualifications)

Copyright waived

Copyright Standards New Zealand

Appendix E: NZS4431:2022 – Appendix A

APPENDIX D – STATEMENT OF SUITABILITY OF ENGINEERED FILL FOR LIGHTWEIGHT STRUCTURES

(Informative)

To: <i>(name and address of local authority)</i>	Selwyn District Council
Development name:	Falcons View Subdivision
Land title(s):	Lot 1 DP 568976 as contained in Record of Title 1024686
Development location/address:	153 Lincoln Rolleston road, Rolleston
Relevant resource consent number(s):	RC225866 and RC225867
Developer's name and company:	Yoursection FV Ltd
Geotechnical designer's name and company:	Charles McDermott of Miyamoto International NZ Ltd
Certifier's name and company:	Charles McDermott of Miyamoto International NZ Ltd
Attachments (give reference numbers):	
(1) Site layout plan(s) Appendix A of this GCR (Miyamoto 2003576-RP-001[A])	
(2) Fill layout plan(s) Appendix A of this GCR (Miyamoto 2003576-RP-001[A])	
(3) Fill section(s)	
(4) Design report Appendix F of this GCR (Miyamoto 2003576-RP-001[A])	
(5) Earthworks completion report, including the following appendices:	
Geotechnical This document comprises Appendix E of the GCR (Miyamoto 2003576-RP-001[A])	
(a) As-built survey;	
(b) Cut-fill plan (with contours);	
(c) Inspection and test plan;	
(d) Earthworks specification;	
(e) All test results;	
(f) All inspection records.	
I confirm I am qualified as a certifier as defined in NZS 4431:2022.	
During this work, I was retained as certifier, and I or my certifier's representative undertook inspections and testing as documented in the attached earthworks completion report.	
I am satisfied that the engineered fill shown in the attached as-built survey was placed, compacted, and tested in accordance with the attached earthworks specification and that all variations and non-compliances have been documented in the earthworks completion report.	
Based on the information available, I certify that, to the best of my knowledge, the intent of the geotechnical designer (as presented in their design, drawings, and earthworks specification) has been achieved.	
The area shown on the as-built survey plan referenced above is considered suitable for development as per NZS 3604. <i>(strike out if not relevant)</i>	
This certification does not remove the necessity for normal inspection and design of foundations as would be made in natural ground.	
Certifier's signature:	Date: 28 November 2023
Certifier's qualifications, professional registration type, and number: BEng(Hons), CMEngNZ, CPEng (1024840)	

Figure 12 – Statement of suitability of engineered fill for lightweight structures

Appendix F: Geotechnical Report for Proposed Plan Change

Geotechnical Report for Proposed Plan Change

Falcons Subdivision Proposed Extension

Issue Date: **25 November 2020**

Miyamoto Ref: **200357-RP-002[A]**

Prepared for: **Yoursection Ltd**

Report Tracking

Revision	Status	Date	Prepared by	Reviewed by
A	FINAL	25 November 2020	C. Gibbens	C. McDermott

Authorisation

Author's Signature		Approver's Signature	
Name	Clem Gibbens	Name	Charles McDermott
Title	Engineering Geologist BSc MSc (Hons) MEngNZ	Title	Associate Geotechnical Engineer BEng (Hons) CEngNZ CPEng

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1. Introduction

Miyamoto International NZ Limited (MINZ) has been engaged by Yoursection Limited to undertake a geotechnical investigation, evaluation and land suitability assessment as part of the proposed land reclassification and plan change required for the proposed extension of the Falcons residential subdivision (encompassing 151 and 153 Lincoln Rolleston Road).

Our assessment comprised the following scope of works:

- Research of available information; including historic reports, the New Zealand Geotechnical Database (NZGD), Selwyn District Council (SDC) and Environment Canterbury (ECan);
- Site walkover inspection of the land;
- Shallow field investigation comprising:
 - Machine excavated trial pits (TP);
 - Dynamic cone penetrometer (DCP) testing.
- Geotechnical Assessment including high-level assessment of the site with regard to the Resource Management Act (RMA) Section 106.

This report presents the findings of our investigation and assessment which were carried out considering the Ministry of Business, Innovation & Employment (MBIE) Guidance documents “Planning and engineering guidance for potentially liquefaction-prone land” - Version 1, dated September 2017, “Repairing and rebuilding houses affected by the Canterbury earthquakes” - Version 3, dated December 2012, and “Earthquake geotechnical engineering practice - Modules 2 & 3”.

It is noted that this report is limited to geotechnical assessment. Advice related to other development requirements (such as roading infrastructure, pavements, services, stormwater management and contaminated land) should be sought from appropriately qualified personnel.

2. Site Description

The site (approximately 25 hectares in area) is located in a rural setting in Rolleston, Selwyn, south of the existing Falcons / Branthwaite residential subdivision, and encompasses the following land parcels (as shown in Figure 1):

- Lot 1 DP 357634;
- Lot 1 DP 50631 BLKS III IV Leeston SD.

The site is predominantly flat with a global elevation difference of 2.0 m to 3.0 m (increasing to the north-west). The land is predominantly grass covered farmland with residential dwellings, workshops and sheep farming buildings currently occupying two relatively small areas of the proposed development area.



Figure 1: Site Location / Layout Plan

3. Data Sources

The following sources of third-party information were considered and are referenced in this report:

- GNS Science - Geological Maps;
- New Zealand Geotechnical Database (NZGD);
- Environment Canterbury (ECan);
- Aurecon (2017). Falcons Landing Geotechnical Subdivision Report;
- Selwyn District Council (SDC);
- Canterbury Maps.

4. Geotechnical Assessment

Geological Setting

The geological map of the area (GNS 1:250,000 QMap) indicates that the site geology is described as 'modern (Quaternary) river floodplain/low-level degradation terraces of unweathered, variably sorted gravel/sand/silt/clay'.

Field Investigations

Miyamoto undertook a site-specific ground investigation on 17 November 2020, comprising:

- 27No. machine excavated trial pits (referenced TP001 to TP027);
- 27No. Dynamic Cone Penetrometer (DCP) tests associated with the above exploratory holes.

In addition to our site-specific investigation we have also utilised available geotechnical information from the surrounding subdivisions and a number of ECan well bores as part of our assessment.

The test locations are shown in Figure 2, the general details of the ground investigations are summarised in Table 1, and the engineering and well bore logs are presented in Appendix A.



Figure 2: Ground Investigation Location Plan

Table 1: Summary of Ground Investigations

Test Ref.	Source	Source Ref.	Test Type	Depth (mbgl)
TP001 to TP027	MINZ	200357	TP / DCP	0.7 to 1.8
Various	Aurecon	254246	TP	1.6 to 1.7
Various	NZGD / Landtech	LTCL18051	TP / DCP	2.1 to 2.6
HA-DCP_128990	NZGD / Davis Ogilvie	39353	HA / DCP	1.2 to 1.7
HA-DCP_27798	NZGD / LDE	10774	TP / DCP	0.8 to 3.0
ECan Well Bores	ECan	Various	Rotary / Percussion / Cable Tool	37.0 to 48.0

Ground Conditions

The ground profile interpreted from the on-site shallow ground investigation, correlated with the available existing data, generally comprises a layer of topsoil (0.2 m to 0.4 m in thickness), overlying low plasticity, firm to stiff Sandy SILT to between 0.4 m and 1.1 mbgl, below which dense to very dense Sandy fine to coarse GRAVEL is present to depth. It is

noted that the upper 0.1 m to 0.2 of the gravel layer is more of a gravelly Sand and a relatively thin layer (0.2 m to 0.4 m) of sand was encountered at isolated locations.

Groundwater

Standing groundwater was not encountered during our site-specific investigation and the soils encountered were dry. Long-term groundwater level monitoring information available from ECan well bores from the surrounding area indicate the groundwater table to average around 10 to 13 mbgl with seasonal fluctuations reaching a shallowest level of ~6 mbgl.

Liquefaction Assessment

The site is located within an area of 'low geotechnical risk' as defined by Selwyn District Council (McCahon, 2013). The site is also located within an area identified as 'Liquefaction damage is unlikely' (2012), and a 'Zone of low liquefaction potential' (2006) as presented on the Canterbury Maps Viewer.

Based on our assessment (including the site-specific ground conditions and groundwater regime) we concur that the risk of damaging effects from liquefaction at the site is low with the seismic performance expected to be equivalent to MBIE Technical Category (TC) 1 as per the MBIE Guidance (2012).

NZS1170.5 Site Sub-soil Class

Based on our geotechnical assessment, geological maps and other available information, NZS1170.5 Site Sub-soil Class D (deep or soft soil site) is considered appropriate for the site.

Flood Hazard

The site is not currently located within one of the Flood Zones identified by Selwyn District Council, however, restrictions around building floor levels must be checked at building consent stage.

5. Development Considerations

At this stage in the project, the future development plans are not defined. However, considering likely residential subdivision similar to that in the local area, the following preliminary guidance is provided:

- Earthworks should be undertaken in general accordance with the requirements of NZS 4431:1989. All unsuitable materials should be stripped from the work areas and stockpiled clear of the operations or removed from site;
- Preliminarily, NZS3604 foundations are considered geotechnically feasible for NZS3604 compliant structures, subject to building-specific geotechnical investigations to assess the available bearing capacity.

It is noted that this report is limited to geotechnical assessment. Advice related to other development requirements (such as roading infrastructure, pavements, services,

stormwater management and contaminated land) should be sought from appropriately qualified personal.

6. Assessment Against RMA Section 106

As per the requirements of Section 106 of the Resource Management Act (RMA) (2017), we have undertaken a high-level assessment of the significant geotechnical hazards that may affect the site. These hazards include, but are not limited to:

- Erosion;
- Falling debris;
- Slippage;
- Subsidence
- Inundation.

At the time of our site visit, there was no evidence of erosion or erosional features on site. The shallow soils could be vulnerable to erosion if the topsoil layer is removed and left unprotected for prolonged periods of time. This can be easily mitigated with appropriate design measures during construction.

Given the proximity of the site to any source, rockfall (falling debris) is not considered a risk to the site and given the site is generally flat with only a minor gradual change in elevation across the site, slope instability (slippage) is not considered to be a risk.

On the basis of our geotechnical assessment herein, we do not consider subsidence (under either static or seismic loading) to be a significant hazard for normal construction (i.e. NZS3604 compliant buildings).

The site is not currently located within one of the Flood Zones identified by Selwyn District Council, however, restrictions around building floor levels must be checked at building consent stage.

Based on our assessment, we consider that the geotechnical hazards may be mitigated to an acceptable standard, provided that the geotechnical recommendations given in this report are followed, and the appropriate engineering measures implemented, we consider that the development is unlikely to be affected nor worsen, accelerate or result in material damage.

7. Limitations

This report is subject to the following limitations:

- This report has been prepared by Miyamoto for the Client for the purpose/s agreed with the Client (Purpose). Miyamoto accepts no responsibility for the validity, appropriateness, sufficiency or consequences of the Client using the report for purposes other than for the Purpose.
- This report is not intended for general publication or circulation. This report is not to be reproduced by the Client except in relation to the Purpose, without Miyamoto's prior written permission. Miyamoto disclaims all risk and all responsibility to any third party.
- This report is provided based on the various assumptions contained in the report.

- Miyamoto’s professional services are performed using a degree of care and skill reasonably exercised by reputable consultants providing the same or similar services as at the date of this report.
- The sub surface information has been obtained from investigation carried out at discrete locations, which by their nature only provide information about a relatively small volume of subsoils. While Miyamoto has taken reasonable skill and care in carrying out the investigation to determine the subsoil condition, the subsoil condition could differ substantially from the results of any sampling investigation. Miyamoto is not responsible for and does not accept any liability in respect of any difference between the actual subsoil conditions and the results of our investigation.
- A change in circumstances, facts, information after the report has been provided may affect the adequacy or accuracy of the report. Miyamoto is not responsible for the adequacy or accuracy of the report as a result of any such changes.
- This report is not to be reproduced, either wholly or in part, without our prior written permission.

If you have any queries or you require any further clarification on any aspects of this report, please do not hesitate to contact Miyamoto International (NZ) Ltd.

References

- Environment Canterbury, 2014. Canterbury Maps Viewer, <http://canterburymaps.govt.nz/Viewer/#webmap>
- Environment Canterbury, Web app viewer - Flood Hazard, <https://ecanmaps.ecan.govt.nz/portal/apps/webappviewer/index.html?id=57c74073c2f14a85ac0caf30073ae48a>
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- GNS Science (2012). Review of liquefaction hazard information in eastern Canterbury, including Christchurch City and parts of Selwyn, Waimakariri and Hurunui Districts, Report No. R12/83
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- Ian MacCahon, 2013. Selwyn District Council 'Area of low geotechnical risk' map.
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- New Zealand Geotechnical Database, 2017. Accessed via Google Earth from <https://www.nzgd.org.nz/>.
- New Zealand Geotechnical Society (NZGS) and Ministry of Business, Innovation and Employment (MBIE) (2016). Earthquake geotechnical engineering practice Module 2: Geotechnical investigations for earthquake engineering, November 2016.
- New Zealand Geotechnical Society (NZGS) and Ministry of Business, Innovation and Employment (MBIE) (2016). Earthquake geotechnical engineering practice Module 3: Identification, assessment and mitigation of liquefaction hazards, May 2016.
- New Zealand Standard NZS1170.5 (2004). Structural Design Actions, Part 5: Earthquake Actions - New Zealand Standard, NZS 2004.
- Selwyn District Council - District Plan Online Maps, <https://eplan.selwyn.govt.nz/eplan/#/Property/7941662>.

Appendices



A. Ground Investigation Data

MINZ site-specific investigation logs

ECan well bore logs

Aurecon 2017 investigation logs (nearby only)

LandTech 2018 investigation logs (nearby only)

Davis Ogilvie 2019 investigation logs (nearby only)

SHALLOW GROUND INVESTIGATION LOG

TP001

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.8 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded			
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW		
							LL	PL	PI	GR	SA	FC					
0.0 - 0.5	6 8 9 9 11 13 20	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)													
0.5 - 1.0				Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium													
1.0 - 1.5				Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular													
1.5 - 2.0				EOH (TARGET STRATA REACHED)													
2.0 - 2.5																	

LEGEND

ABBREVIATIONS

DCP DYNAMIC CONE PENETROMETER
 HA HAND AUGER
 SV SHEAR VANE
 TP TEST PIT
 GWL GROUNDWATER LEVEL

N/E NOT ENCOUNTERED
 UTP UNABLE TO PENETRATE
 EOH END OF HOLE
 UW UNIT WEIGHT (kN/m³)
 mbgl METERS BELOW GROUND LEVEL

LL LIQUID LIMIT
 PL PLASTIC LIMIT
 PI PLASTICITY INDEX
 WC WATER CONTENT

GR GRAVEL
 SA SAND
 FC FINES CONTENT
 STANDING GWL

NOTES

SHALLOW GROUND INVESTIGATION LOG

TP002

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 1.0 mbgl
PROCESSED BY: CG	TESTING METHOD: TP + DCP
LOCATION: REFER TO SITE PLAN	GROUNDWATER LEVEL: N/E

HOLE DIAMETER: 50 mm

SHEAR VANE NUMBER: -

This report may only be reproduced in full

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded	
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW
							LL	PL	PI	GR	SA	FC			
0.0 - 0.1	6	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)	[Cross-hatch pattern]										
0.1 - 0.2	6			Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium	[x pattern]										
0.2 - 0.3	13														
0.3 - 0.4	12														
0.4 - 0.5	9														
0.5 - 0.6	7														
0.6 - 0.7	7														
0.7 - 0.8	6														
0.8 - 0.9	7														
0.9 - 1.0	11				Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular	[Dotted pattern]									
1.0 - 1.1				EOH (TARGET STRATA REACHED)											

Weight Bouncing

LEGEND

ABBREVIATIONS

DCP DYNAMIC CONE PENETROMETER	N/E NOT ENCOUNTERED	LL LIQUID LIMIT	GR GRAVEL
HA HAND AUGER	UTP UNABLE TO PENETRATE	PL PLASTIC LIMIT	SA SAND
SV SHEAR VANE	EOH END OF HOLE	PI PLASTICITY INDEX	FC FINES CONTENT
TP TEST PIT	UW UNIT WEIGHT (kN/m ³)	WC WATER CONTENT	STANDING GWL
GWL GROUNDWATER LEVEL	mbgl METERS BELOW GROUND LEVEL		

NOTES

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SHALLOW GROUND INVESTIGATION LOG

TP003

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.7 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded				
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW			
							LL	PL	PI	GR	SA	FC						
0.0 - 0.5	5 5 11 12 15 20	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)														
0.5 - 0.7				Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium														
0.7 - 1.0				Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular														
1.0 - 2.5			EOH (TARGET STRATA REACHED)															

LEGEND

ABBREVIATIONS

DCP DYNAMIC CONE PENETROMETER
HA HAND AUGER
SV SHEAR VANE
TP TEST PIT
GWL GROUNDWATER LEVEL

N/E NOT ENCOUNTERED
UTP UNABLE TO PENETRATE
EOH END OF HOLE
UW UNIT WEIGHT (kN/m³)
mbgl METERS BELOW GROUND LEVEL

LL LIQUID LIMIT
PL PLASTIC LIMIT
PI PLASTICITY INDEX
WC WATER CONTENT
GR GRAVEL
SA SAND
FC FINES CONTENT
 STANDING GWL

NOTES

SHALLOW GROUND INVESTIGATION LOG

TP004

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 1.2 mbgl
PROCESSED BY: CG	TESTING METHOD: TP + DCP
LOCATION: REFER TO SITE PLAN	GROUNDWATER LEVEL: N/E
HOLE DIAMETER: 50 mm	
SHEAR VANE NUMBER: -	
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded		
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW	
							LL	PL	PI	GR	SA	FC				
4	4	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)												
5	5			Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium												
7	7															
10	10															
11	11				SAND; fine to medium, yellow-brown, dry											
10	10															
9	9			Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular												
10	10															
5	5			EOH (TARGET STRATA REACHED)												
5	5															
11	11															

LEGEND

ABBREVIATIONS

DCP	DYNAMIC CONE PENETROMETER	N/E	NOT ENCOUNTERED	LL	LIQUID LIMIT	GR	GRAVEL
HA	HAND AUGER	UTP	UNABLE TO PENETRATE	PL	PLASTIC LIMIT	SA	SAND
SV	SHEAR VANE	EOH	END OF HOLE	PI	PLASTICITY INDEX	FC	FINES CONTENT
TP	TEST PIT	UW	UNIT WEIGHT (kN/m ³)	WC	WATER CONTENT		STANDING GWL
GWL	GROUNDWATER LEVEL	mbgl	METERS BELOW GROUND LEVEL				

NOTES

--

SHALLOW GROUND INVESTIGATION LOG

TP005

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.6 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded				
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW			
							LL	PL	PI	GR	SA	FC						
5	5	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)														
6	6			Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium														
7	7			Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular														
8	8			EOH (TARGET STRATA REACHED)														
15	15																	
20	20																	

LEGEND

ABBREVIATIONS

DCP	DYNAMIC CONE PENETROMETER	N/E	NOT ENCOUNTERED	LL	LIQUID LIMIT	GR	GRAVEL
HA	HAND AUGER	UTP	UNABLE TO PENETRATE	PL	PLASTIC LIMIT	SA	SAND
SV	SHEAR VANE	EOH	END OF HOLE	PI	PLASTICITY INDEX	FC	FINES CONTENT
TP	TEST PIT	UW	UNIT WEIGHT (kN/m ³)	WC	WATER CONTENT		STANDING GWL
GWL	GROUNDWATER LEVEL	mbgl	METERS BELOW GROUND LEVEL				

NOTES

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SHALLOW GROUND INVESTIGATION LOG

TP006

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 1.3 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded		
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW	
							LL	PL	PI	GR	SA	FC				
0.0 - 0.5	5	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)	[Cross-hatch pattern]											
0.5 - 1.0	5, 4, 4, 6, 6, 5, 5, 4			Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium	[X pattern]											
1.0 - 1.5	4, 5, 6			SAND; fine to medium, yellow-brown, dry	[Dotted pattern]											
1.5 - 2.0	15			Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular	[Dotted pattern]											
2.0 - 2.5				EOH (TARGET STRATA REACHED)												

LEGEND

ABBREVIATIONS

DCP DYNAMIC CONE PENETROMETER	N/E NOT ENCOUNTERED	LL LIQUID LIMIT	GR GRAVEL
HA HAND AUGER	UTP UNABLE TO PENETRATE	PL PLASTIC LIMIT	SA SAND
SV SHEAR VANE	EOH END OF HOLE	PI PLASTICITY INDEX	FC FINES CONTENT
TP TEST PIT	UW UNIT WEIGHT (kN/m ³)	WC WATER CONTENT	STANDING GWL
GWL GROUNDWATER LEVEL	mbgl METERS BELOW GROUND LEVEL		

NOTES

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SHALLOW GROUND INVESTIGATION LOG

TP007

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.6 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded	
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW
							LL	PL	PI	GR	SA	FC			
0.5	5 6 8 20 Weight Bouncing	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)											
				Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium											
				Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular											
				EOH (TARGET STRATA REACHED)											

LEGEND

ABBREVIATIONS						NOTES	
DCP	DYNAMIC CONE PENETROMETER	N/E	NOT ENCOUNTERED	LL	LIQUID LIMIT	GR	GRAVEL
HA	HAND AUGER	UTP	UNABLE TO PENETRATE	PL	PLASTIC LIMIT	SA	SAND
SV	SHEAR VANE	EOH	END OF HOLE	PI	PLASTICITY INDEX	FC	FINES CONTENT
TP	TEST PIT	UW	UNIT WEIGHT (kN/m³)	WC	WATER CONTENT		STANDING GWL
GWL	GROUNDWATER LEVEL	mbgl	METERS BELOW GROUND LEVEL				

SHALLOW GROUND INVESTIGATION LOG

TP008

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.5 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded		
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW	
							LL	PL	PI	GR	SA	FC				
0.0 - 0.5	5 7 8 17 Weight Bouncing			SILT; non-plastic, dark brown, dry (TOPSOIL)												
0.5 - 0.7				Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium												
0.7 - 0.5				Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular												
0.5 - 2.5		NOT ENCOUNTERED		EOH (TARGET STRATA REACHED)												

LEGEND

ABBREVIATIONS						NOTES	
DCP	DYNAMIC CONE PENETROMETER	N/E	NOT ENCOUNTERED	LL	LIQUID LIMIT	GR	GRAVEL
HA	HAND AUGER	UTP	UNABLE TO PENETRATE	PL	PLASTIC LIMIT	SA	SAND
SV	SHEAR VANE	EOH	END OF HOLE	PI	PLASTICITY INDEX	FC	FINES CONTENT
TP	TEST PIT	UW	UNIT WEIGHT (kN/m³)	WC	WATER CONTENT		STANDING GWL
GWL	GROUNDWATER LEVEL	mbgl	METERS BELOW GROUND LEVEL				

SHALLOW GROUND INVESTIGATION LOG

TP009

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.7 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded			
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW		
							LL	PL	PI	GR	SA	FC					
5	5	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)													
4	4																
5	5																
6	6																
0.5	7			Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium													
7	8																
8	7			Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular													
1.0	16			EOH (TARGET STRATA REACHED)													
1.0	Weight Bouncing																
1.5																	
2.0																	
2.5																	

LEGEND

ABBREVIATIONS

DCP	DYNAMIC CONE PENETROMETER	N/E	NOT ENCOUNTERED	LL	LIQUID LIMIT	GR	GRAVEL
HA	HAND AUGER	UTP	UNABLE TO PENETRATE	PL	PLASTIC LIMIT	SA	SAND
SV	SHEAR VANE	EOH	END OF HOLE	PI	PLASTICITY INDEX	FC	FINES CONTENT
TP	TEST PIT	UW	UNIT WEIGHT (kN/m ³)	WC	WATER CONTENT		STANDING GWL
GWL	GROUNDWATER LEVEL	mbgl	METERS BELOW GROUND LEVEL				

NOTES

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SHALLOW GROUND INVESTIGATION LOG

TP010

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.7 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded	
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW
							LL	PL	PI	GR	SA	FC			
0.0 - 0.1	5	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)											
0.1 - 0.2	4														
0.2 - 0.3	4														
0.3 - 0.4	2				Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium										
0.4 - 0.5	7														
0.5 - 0.6	16			Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular											
0.6 - 0.7				EOH (TARGET STRATA REACHED)											
0.7 - 0.8															
0.8 - 0.9															
0.9 - 1.0															
1.0 - 1.1															
1.1 - 1.2															
1.2 - 1.3															
1.3 - 1.4															
1.4 - 1.5															
1.5 - 1.6															
1.6 - 1.7															
1.7 - 1.8															
1.8 - 1.9															
1.9 - 2.0															
2.0 - 2.1															
2.1 - 2.2															
2.2 - 2.3															
2.3 - 2.4															
2.4 - 2.5															
2.5 - 2.6															
2.6 - 2.7															
2.7 - 2.8															
2.8 - 2.9															
2.9 - 3.0															

LEGEND

ABBREVIATIONS

DCP	DYNAMIC CONE PENETROMETER	N/E	NOT ENCOUNTERED	LL	LIQUID LIMIT	GR	GRAVEL
HA	HAND AUGER	UTP	UNABLE TO PENETRATE	PL	PLASTIC LIMIT	SA	SAND
SV	SHEAR VANE	EOH	END OF HOLE	PI	PLASTICITY INDEX	FC	FINES CONTENT
TP	TEST PIT	UW	UNIT WEIGHT (kN/m ³)	WC	WATER CONTENT		STANDING GWL
GWL	GROUNDWATER LEVEL	mbgl	METERS BELOW GROUND LEVEL				

NOTES

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SHALLOW GROUND INVESTIGATION LOG

TP011

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.7 mbgl
PROCESSED BY: CG	TESTING METHOD: TP + DCP
LOCATION: REFER TO SITE PLAN	GROUNDWATER LEVEL: N/E

HOLE DIAMETER: 50 mm

SHEAR VANE NUMBER: -

This report may only be reproduced in full

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded	
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW
							LL	PL	PI	GR	SA	FC			
0.0 - 0.1	8	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)											
0.1 - 0.2	9														
0.2 - 0.3	7														
0.3 - 0.4	7														
0.4 - 0.5	11			Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium											
0.5 - 0.6	20			Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular											
0.6 - 0.7				EOH (TARGET STRATA REACHED)											

Weight Bouncing

LEGEND

ABBREVIATIONS

DCP	DYNAMIC CONE PENETROMETER	N/E	NOT ENCOUNTERED	LL	LIQUID LIMIT	GR	GRAVEL
HA	HAND AUGER	UTP	UNABLE TO PENETRATE	PL	PLASTIC LIMIT	SA	SAND
SV	SHEAR VANE	EOH	END OF HOLE	PI	PLASTICITY INDEX	FC	FINES CONTENT
TP	TEST PIT	UW	UNIT WEIGHT (kN/m ³)	WC	WATER CONTENT		STANDING GWL
GWL	GROUNDWATER LEVEL	mbgl	METERS BELOW GROUND LEVEL				

NOTES

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SHALLOW GROUND INVESTIGATION LOG

TP012

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.8 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded			
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW		
							LL	PL	PI	GR	SA	FC					
0.0 - 0.1	5	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)	[Cross-hatch pattern]												
0.1 - 0.2	4			Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium	[X pattern]												
0.2 - 0.3	9																
0.3 - 0.4	13																
0.4 - 0.5	11																
0.5 - 0.6	7																
0.6 - 0.7	8																
0.7 - 0.8	20			Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular	[Gravel pattern]												
0.8 - 1.0	Weight Bouncing			EOH (TARGET STRATA REACHED)													

LEGEND

ABBREVIATIONS						NOTES	
DCP	DYNAMIC CONE PENETROMETER	N/E	NOT ENCOUNTERED	LL	LIQUID LIMIT	GR	GRAVEL
HA	HAND AUGER	UTP	UNABLE TO PENETRATE	PL	PLASTIC LIMIT	SA	SAND
SV	SHEAR VANE	EOH	END OF HOLE	PI	PLASTICITY INDEX	FC	FINES CONTENT
TP	TEST PIT	UW	UNIT WEIGHT (kN/m ³)	WC	WATER CONTENT		STANDING GWL
GWL	GROUNDWATER LEVEL	mbgl	METERS BELOW GROUND LEVEL				

SHALLOW GROUND INVESTIGATION LOG

TP013

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.9 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded	
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW
							LL	PL	PI	GR	SA	FC			
0.0 - 0.1	6	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)											
0.1 - 0.2	5			Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium											
0.2 - 0.3	8			SAND; fine to medium, yellow-brown, dry											
0.3 - 0.4	15			Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular											
0.4 - 0.5	21		EOH (TARGET STRATA REACHED)												

LEGEND

ABBREVIATIONS

DCP DYNAMIC CONE PENETROMETER
 HA HAND AUGER
 SV SHEAR VANE
 TP TEST PIT
 GWL GROUNDWATER LEVEL

N/E NOT ENCOUNTERED
 UTP UNABLE TO PENETRATE
 EOH END OF HOLE
 UW UNIT WEIGHT (kN/m³)
 mbgl METERS BELOW GROUND LEVEL

LL LIQUID LIMIT
 PL PLASTIC LIMIT
 PI PLASTICITY INDEX
 WC WATER CONTENT

GR GRAVEL
 SA SAND
 FC FINES CONTENT
 STANDING GWL

NOTES

SHALLOW GROUND INVESTIGATION LOG

TP014

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.5 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded	
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW
							LL	PL	PI	GR	SA	FC			
0.0 - 0.08	8	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)											
0.08 - 0.15	15			Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium											
0.15 - 0.5	15			Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular											
0.5 - 0.5	Weight Bouncing			EOH (TARGET STRATA REACHED)											

LEGEND

ABBREVIATIONS

DCP DYNAMIC CONE PENETROMETER
 HA HAND AUGER
 SV SHEAR VANE
 TP TEST PIT
 GWL GROUNDWATER LEVEL

N/E NOT ENCOUNTERED
 UTP UNABLE TO PENETRATE
 EOH END OF HOLE
 UW UNIT WEIGHT (kN/m³)
 mbgl METERS BELOW GROUND LEVEL

LL LIQUID LIMIT
 PL PLASTIC LIMIT
 PI PLASTICITY INDEX
 WC WATER CONTENT

GR GRAVEL
 SA SAND
 FC FINES CONTENT
 STANDING GWL

NOTES

SHALLOW GROUND INVESTIGATION LOG

TP015

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 1.0 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded	
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW
							LL	PL	PI	GR	SA	FC			
0.0 - 0.1	7	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)											
0.1 - 0.2	6			Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium											
0.2 - 0.3	4														
0.3 - 0.4	4														
0.4 - 0.5	5			Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular											
0.5 - 0.6	6														
0.6 - 0.7	4														
0.7 - 0.8	6			EOH (TARGET STRATA REACHED)											
0.8 - 0.9	9														
0.9 - 1.0	20														
1.0 - 1.1	Weight Bouncing														

LEGEND

ABBREVIATIONS						NOTES	
DCP	DYNAMIC CONE PENETROMETER	N/E	NOT ENCOUNTERED	LL	LIQUID LIMIT	GR	GRAVEL
HA	HAND AUGER	UTP	UNABLE TO PENETRATE	PL	PLASTIC LIMIT	SA	SAND
SV	SHEAR VANE	EOH	END OF HOLE	PI	PLASTICITY INDEX	FC	FINES CONTENT
TP	TEST PIT	UW	UNIT WEIGHT (kN/m ³)	WC	WATER CONTENT		STANDING GWL
GWL	GROUNDWATER LEVEL	mbgl	METERS BELOW GROUND LEVEL				

SHALLOW GROUND INVESTIGATION LOG

TP016

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.8 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded	
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW
							LL	PL	PI	GR	SA	FC			
0.0 - 0.5	5 7 11 20 Weight Bouncing	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)											
0.5 - 1.0				Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium											
1.0 - 1.1				Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular											
1.1 - 2.5			EOH (TARGET STRATA REACHED)												

LEGEND

ABBREVIATIONS						NOTES	
DCP	DYNAMIC CONE PENETROMETER	N/E	NOT ENCOUNTERED	LL	LIQUID LIMIT	GR	GRAVEL
HA	HAND AUGER	UTP	UNABLE TO PENETRATE	PL	PLASTIC LIMIT	SA	SAND
SV	SHEAR VANE	EOH	END OF HOLE	PI	PLASTICITY INDEX	FC	FINES CONTENT
TP	TEST PIT	UW	UNIT WEIGHT (kN/m ³)	WC	WATER CONTENT		STANDING GWL
GWL	GROUNDWATER LEVEL	mbgl	METERS BELOW GROUND LEVEL				

SHALLOW GROUND INVESTIGATION LOG

TP017

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.8 mbgl
PROCESSED BY: CG	TESTING METHOD: TP + DCP
LOCATION: REFER TO SITE PLAN	GROUNDWATER LEVEL: N/E

HOLE DIAMETER: 50 mm

SHEAR VANE NUMBER: -

This report may only be reproduced in full

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded	
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW
							LL	PL	PI	GR	SA	FC			
0.0 - 0.1	4	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)											
0.1 - 0.2	3			Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium											
0.2 - 0.3	3														
0.3 - 0.4	4														
0.4 - 0.5	4			Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular											
0.5 - 0.6	4														
0.6 - 0.7	4			EOH (TARGET STRATA REACHED)											
0.7 - 0.8	5														
0.8 - 0.9	20														
0.9 - 1.0	Weight Bouncing														

LEGEND

ABBREVIATIONS

DCP	DYNAMIC CONE PENETROMETER	N/E	NOT ENCOUNTERED	LL	LIQUID LIMIT	GR	GRAVEL
HA	HAND AUGER	UTP	UNABLE TO PENETRATE	PL	PLASTIC LIMIT	SA	SAND
SV	SHEAR VANE	EOH	END OF HOLE	PI	PLASTICITY INDEX	FC	FINES CONTENT
TP	TEST PIT	UW	UNIT WEIGHT (kN/m ³)	WC	WATER CONTENT		STANDING GWL
GWL	GROUNDWATER LEVEL	mbgl	METERS BELOW GROUND LEVEL				

NOTES

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SHALLOW GROUND INVESTIGATION LOG

TP018

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.4 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded			
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW		
							LL	PL	PI	GR	SA	FC					
0.0 - 0.5	5 8 20 Weight Bouncing	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)													
0.5 - 0.6				Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular													
0.6 - 2.5				EOH (TARGET STRATA REACHED)													

LEGEND

ABBREVIATIONS

DCP DYNAMIC CONE PENETROMETER
 HA HAND AUGER
 SV SHEAR VANE
 TP TEST PIT
 GWL GROUNDWATER LEVEL

N/E NOT ENCOUNTERED
 UTP UNABLE TO PENETRATE
 EOH END OF HOLE
 UW UNIT WEIGHT (kN/m³)
 mbgl METERS BELOW GROUND LEVEL

LL LIQUID LIMIT
 PL PLASTIC LIMIT
 PI PLASTICITY INDEX
 WC WATER CONTENT

GR GRAVEL
 SA SAND
 FC FINES CONTENT
 STANDING GWL

NOTES

SHALLOW GROUND INVESTIGATION LOG

TP019

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.4 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded	
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW
							LL	PL	PI	GR	SA	FC			
0.0 - 0.1	5	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)											
0.1 - 0.2	5			Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular											
0.2 - 0.4	20		EOH (TARGET STRATA REACHED)												
0.4 - 1.0	Weight Bouncing														
1.0 - 2.5															

LEGEND

ABBREVIATIONS

DCP	DYNAMIC CONE PENETROMETER	N/E	NOT ENCOUNTERED	LL	LIQUID LIMIT	GR	GRAVEL
HA	HAND AUGER	UTP	UNABLE TO PENETRATE	PL	PLASTIC LIMIT	SA	SAND
SV	SHEAR VANE	EOH	END OF HOLE	PI	PLASTICITY INDEX	FC	FINES CONTENT
TP	TEST PIT	UW	UNIT WEIGHT (kN/m ³)	WC	WATER CONTENT		STANDING GWL
GWL	GROUNDWATER LEVEL	mbgl	METERS BELOW GROUND LEVEL				

NOTES

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SHALLOW GROUND INVESTIGATION LOG

TP020

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 1.2 mbgl
PROCESSED BY: CG	TESTING METHOD: TP + DCP
LOCATION: REFER TO SITE PLAN	GROUNDWATER LEVEL: N/E

HOLE DIAMETER: 50 mm
SHEAR VANE NUMBER: -
This report may only be reproduced in full

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded	
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW
							LL	PL	PI	GR	SA	FC			
0.0 - 0.25	4	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)	[Cross-hatch pattern]										
0.25 - 0.5	3			Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium	[X pattern]										
0.5 - 0.75	4														
0.75 - 1.0	4														
1.0 - 1.25	7				[X pattern]										
1.25 - 1.5	16				[X pattern]										
1.5 - 1.75	17				[X pattern]										
1.75 - 2.0	13				[X pattern]										
2.0 - 2.25	20				[X pattern]										
2.25 - 2.5					[Dotted pattern]										
2.5 - 2.75															
2.75 - 3.0															
3.0 - 3.25															
3.25 - 3.5															
3.5 - 3.75															
3.75 - 4.0															
4.0 - 4.25															
4.25 - 4.5															
4.5 - 4.75															
4.75 - 5.0															
5.0 - 5.25															
5.25 - 5.5															
5.5 - 5.75															
5.75 - 6.0															
6.0 - 6.25															
6.25 - 6.5															
6.5 - 6.75															
6.75 - 7.0															
7.0 - 7.25															
7.25 - 7.5															
7.5 - 7.75															
7.75 - 8.0															
8.0 - 8.25															
8.25 - 8.5															
8.5 - 8.75															
8.75 - 9.0															
9.0 - 9.25															
9.25 - 9.5															
9.5 - 9.75															
9.75 - 10.0															

Weight Bouncing

NOT ENCOUNTERED

Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular
EOH (TARGET STRATA REACHED)

LEGEND

ABBREVIATIONS

DCP	DYNAMIC CONE PENETROMETER	N/E	NOT ENCOUNTERED	LL	LIQUID LIMIT	GR	GRAVEL
HA	HAND AUGER	UTP	UNABLE TO PENETRATE	PL	PLASTIC LIMIT	SA	SAND
SV	SHEAR VANE	EOH	END OF HOLE	PI	PLASTICITY INDEX	FC	FINES CONTENT
TP	TEST PIT	UW	UNIT WEIGHT (kN/m ³)	WC	WATER CONTENT		STANDING GWL
GWL	GROUNDWATER LEVEL	mbgl	METERS BELOW GROUND LEVEL				

NOTES

SHALLOW GROUND INVESTIGATION LOG

TP021

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.9 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded				
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW			
							LL	PL	PI	GR	SA	FC						
0.0 - 0.5	5 4 5 5 6 5 6 6	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)														
0.5 - 1.0				Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium														
1.0 - 1.7	17 Weight Bouncing			Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular														
1.7 - 2.5			EOH (TARGET STRATA REACHED)															

LEGEND

ABBREVIATIONS

DCP DYNAMIC CONE PENETROMETER
 HA HAND AUGER
 SV SHEAR VANE
 TP TEST PIT
 GWL GROUNDWATER LEVEL

N/E NOT ENCOUNTERED
 UTP UNABLE TO PENETRATE
 EOH END OF HOLE
 UW UNIT WEIGHT (kN/m³)
 mbgl METERS BELOW GROUND LEVEL

LL LIQUID LIMIT
 PL PLASTIC LIMIT
 PI PLASTICITY INDEX
 WC WATER CONTENT

GR GRAVEL
 SA SAND
 FC FINES CONTENT
 STANDING GWL

NOTES

SHALLOW GROUND INVESTIGATION LOG

TP022

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.6 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded	
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW
							LL	PL	PI	GR	SA	FC			
0.3	5	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)											
0.4	4			Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium											
0.5	5														
0.6	7				Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular										
0.7	20														
<p>Weight Bouncing</p> <p>EOH (TARGET STRATA REACHED)</p>															

LEGEND

ABBREVIATIONS						NOTES	
DCP	DYNAMIC CONE PENETROMETER	N/E	NOT ENCOUNTERED	LL	LIQUID LIMIT	GR	GRAVEL
HA	HAND AUGER	UTP	UNABLE TO PENETRATE	PL	PLASTIC LIMIT	SA	SAND
SV	SHEAR VANE	EOH	END OF HOLE	PI	PLASTICITY INDEX	FC	FINES CONTENT
TP	TEST PIT	UW	UNIT WEIGHT (kN/m ³)	WC	WATER CONTENT		STANDING GWL
GWL	GROUNDWATER LEVEL	mbgl	METERS BELOW GROUND LEVEL				

SHALLOW GROUND INVESTIGATION LOG

TP023

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.9 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded	
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW
							LL	PL	PI	GR	SA	FC			
0.0 - 0.1	7	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)											
0.1 - 0.2	7			Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium											
0.2 - 0.3	8														
0.3 - 0.4	15				Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular										
0.4 - 0.5	13														
0.5 - 0.6	8			Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular											
0.6 - 0.7	13														
0.7 - 1.0	Weight Bouncing			EOH (TARGET STRATA REACHED)											
1.0 - 1.5															
1.5 - 2.0															
2.0 - 2.5															

LEGEND

ABBREVIATIONS

DCP DYNAMIC CONE PENETROMETER
HA HAND AUGER
SV SHEAR VANE
TP TEST PIT
GWL GROUNDWATER LEVEL

N/E NOT ENCOUNTERED
UTP UNABLE TO PENETRATE
EOH END OF HOLE
UW UNIT WEIGHT (kN/m³)
mbgl METERS BELOW GROUND LEVEL

LL LIQUID LIMIT
PL PLASTIC LIMIT
PI PLASTICITY INDEX
WC WATER CONTENT
GR GRAVEL
SA SAND
FC FINES CONTENT
 STANDING GWL

NOTES

SHALLOW GROUND INVESTIGATION LOG

TP024

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.9 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded	
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW
							LL	PL	PI	GR	SA	FC			
0.0 - 0.3	3	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)											
0.3 - 0.4	4														
0.4 - 0.5	3														
0.5 - 0.7	5				Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium										
0.7 - 0.9	7														
0.9 - 1.0	15			Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular											
1.0 - 2.5				EOH (TARGET STRATA REACHED)											

LEGEND

ABBREVIATIONS

DCP	DYNAMIC CONE PENETROMETER	N/E	NOT ENCOUNTERED	LL	LIQUID LIMIT	GR	GRAVEL
HA	HAND AUGER	UTP	UNABLE TO PENETRATE	PL	PLASTIC LIMIT	SA	SAND
SV	SHEAR VANE	EOH	END OF HOLE	PI	PLASTICITY INDEX	FC	FINES CONTENT
TP	TEST PIT	UW	UNIT WEIGHT (kN/m ³)	WC	WATER CONTENT		STANDING GWL
GWL	GROUNDWATER LEVEL	mbgl	METERS BELOW GROUND LEVEL				

NOTES

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SHALLOW GROUND INVESTIGATION LOG

TP025

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.5 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded	
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW
							LL	PL	PI	GR	SA	FC			
0.0 - 0.1	4	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)											
0.1 - 0.2	4			Sandy SILT; low plasticity, yellow-brown, dry, sand is fine to medium											
0.2 - 0.3	6			Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular											
0.3 - 0.4	15			EOH (TARGET STRATA REACHED)											
0.4 - 0.5	Weight Bouncing														
0.5 - 2.5															

LEGEND

ABBREVIATIONS

DCP	DYNAMIC CONE PENETROMETER	N/E	NOT ENCOUNTERED	LL	LIQUID LIMIT	GR	GRAVEL
HA	HAND AUGER	UTP	UNABLE TO PENETRATE	PL	PLASTIC LIMIT	SA	SAND
SV	SHEAR VANE	EOH	END OF HOLE	PI	PLASTICITY INDEX	FC	FINES CONTENT
TP	TEST PIT	UW	UNIT WEIGHT (kN/m ³)	WC	WATER CONTENT		STANDING GWL
GWL	GROUNDWATER LEVEL	mbgl	METERS BELOW GROUND LEVEL				

NOTES

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SHALLOW GROUND INVESTIGATION LOG

TP026

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.4 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded			
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW		
							LL	PL	PI	GR	SA	FC					
0.0 - 0.1	3	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)													
0.1 - 0.2	7				Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular												
0.2 - 0.3	15																
0.3 - 0.4	20																
0.4	Weight Bouncing			EOH (TARGET STRATA REACHED)													

LEGEND

ABBREVIATIONS

DCP DYNAMIC CONE PENETROMETER
HA HAND AUGER
SV SHEAR VANE
TP TEST PIT
GWL GROUNDWATER LEVEL

N/E NOT ENCOUNTERED
UTP UNABLE TO PENETRATE
EOH END OF HOLE
UW UNIT WEIGHT (kN/m³)
mbgl METERS BELOW GROUND LEVEL

LL LIQUID LIMIT
PL PLASTIC LIMIT
PI PLASTICITY INDEX
WC WATER CONTENT

GR GRAVEL
SA SAND
FC FINES CONTENT
 STANDING GWL

NOTES

SHALLOW GROUND INVESTIGATION LOG

TP027

PROJECT: 151 & 153 Lincoln Rolleston Road, Rolleston	
LOGGED BY: CG	TOTAL TESTING DEPTH: 0.3 mbgl
PROCESSED BY: CG	HOLE DIAMETER: 50 mm
LOCATION: REFER TO SITE PLAN	TESTING METHOD: TP + DCP
	SHEAR VANE NUMBER: -
	GROUNDWATER LEVEL: N/E
This report may only be reproduced in full	

Depth (m)	DCP Test Results (Blows per 100mm)	GWL	Soil Description			Sample Taken	Lab Testing							Shear Vane Reading (kPa) peak/remoulded			
			USC	Soil Characteristics	Graphic Log		Atterberg Limits			Grain Size			WC (%)		UW		
							LL	PL	PI	GR	SA	FC					
0.0 - 0.1	5	NOT ENCOUNTERED		SILT; non-plastic, dark brown, dry (TOPSOIL)													
0.1 - 0.2	5			Gravelly fine to coarse SAND / Sandy Gravel; fine to coarse, grey, dry, gravel is subrounded to subangular													
0.2 - 0.3	15			EOH (TARGET STRATA REACHED)													

LEGEND

ABBREVIATIONS

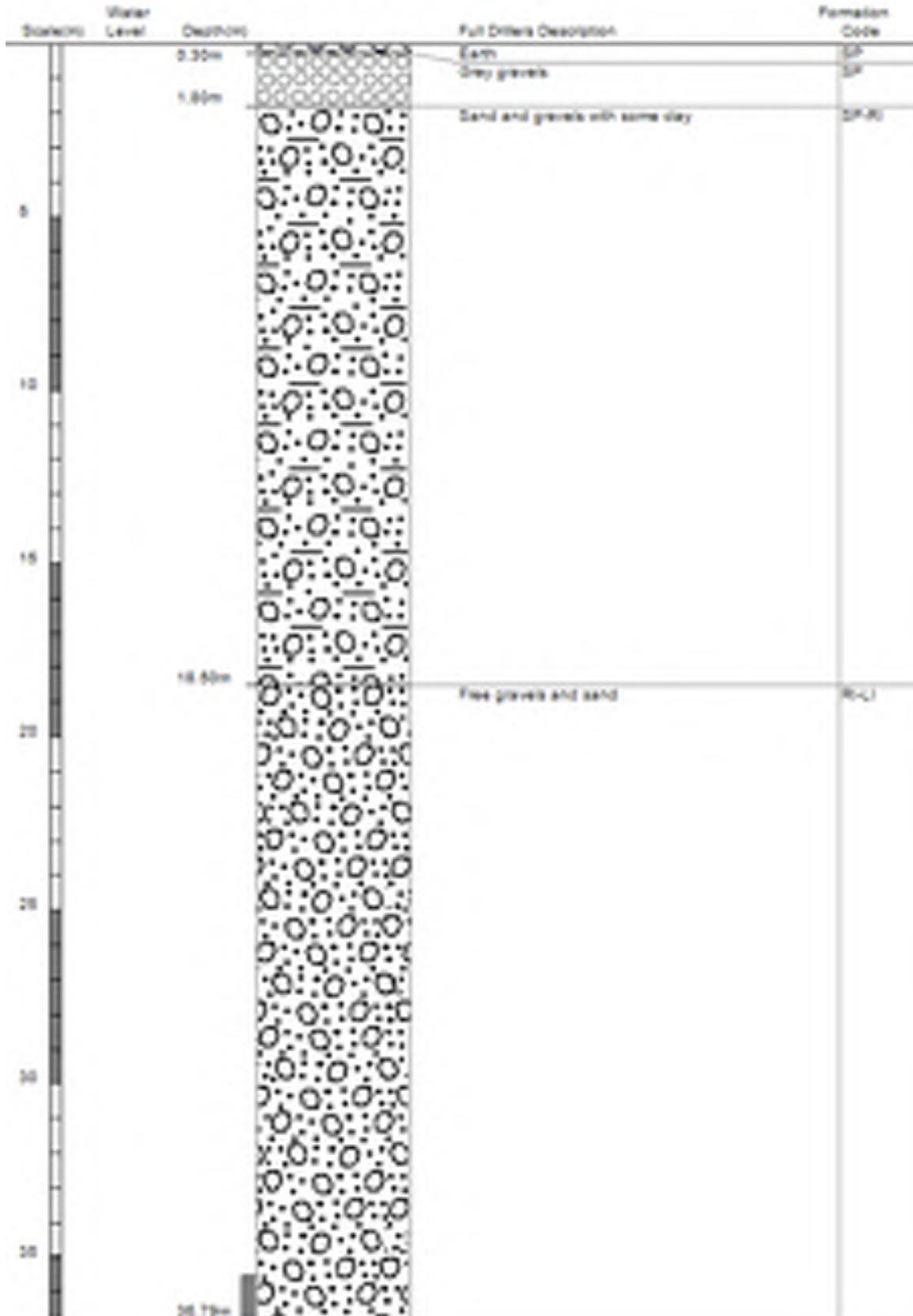
DCP	DYNAMIC CONE PENETROMETER	N/E	NOT ENCOUNTERED	LL	LIQUID LIMIT	GR	GRAVEL
HA	HAND AUGER	UTP	UNABLE TO PENETRATE	PL	PLASTIC LIMIT	SA	SAND
SV	SHEAR VANE	EOH	END OF HOLE	PI	PLASTICITY INDEX	FC	FINES CONTENT
TP	TEST PIT	UW	UNIT WEIGHT (kN/m ³)	WC	WATER CONTENT		STANDING GWL
GWL	GROUNDWATER LEVEL	mbgl	METERS BELOW GROUND LEVEL				

NOTES

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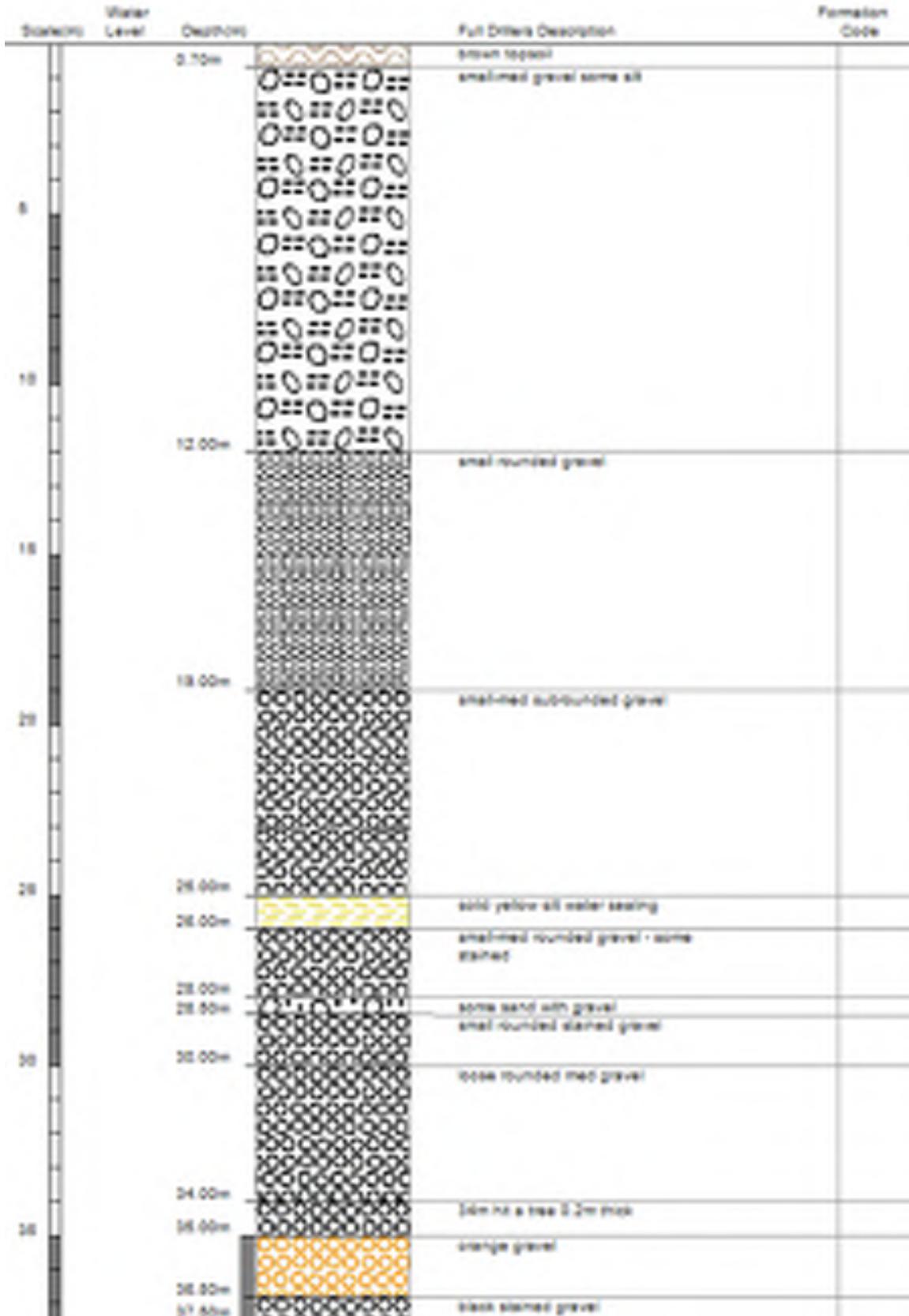
Borelog for well M36/3868

Grid Reference (NZTM): 1552494 mE, 5971203 mN
 Location Accuracy: 10 - 50m
 Ground Level Altitude: 38.8 m -MSD Accuracy: < 2.5 m
 Driller: McMillan Drilling Ltd
 Drill Method: Rotary/Perussion
 Borelog Depth: 36.8 m Drill Date: 18-Jan-1988



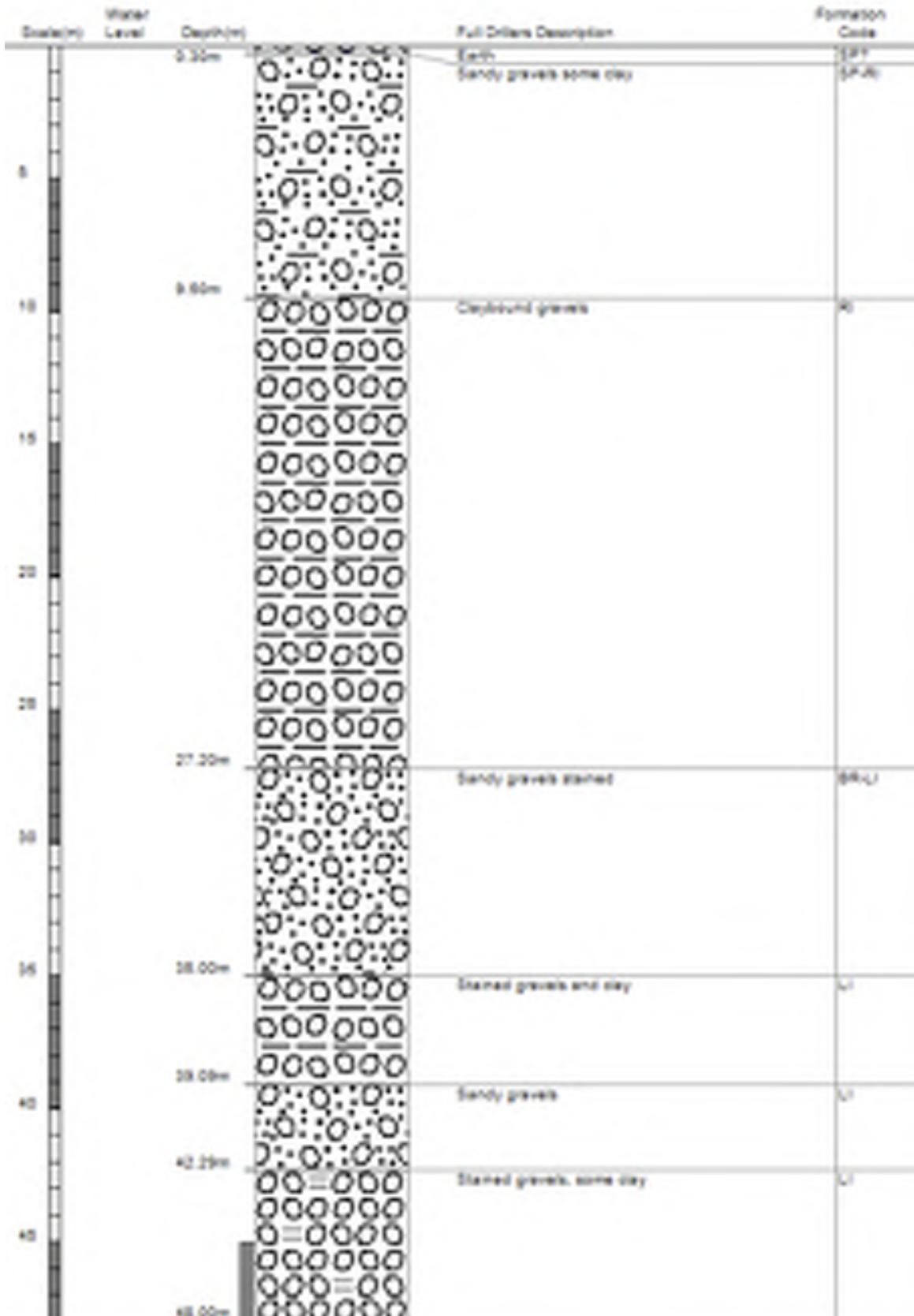
Borelog for well M36/7976

Grid Reference (NZTM): 1552317 mE, 5171001 mN
 Location Accuracy: 50 - 300m
 Ground Level Altitude: 37.7 m +MSD Accuracy: < 2.5 m
 Driller: Dynes Road Drilling
 Drill Method: Cable Tool
 Borelog Depth: 37.5 m Drill Date: 05-Sep-2005



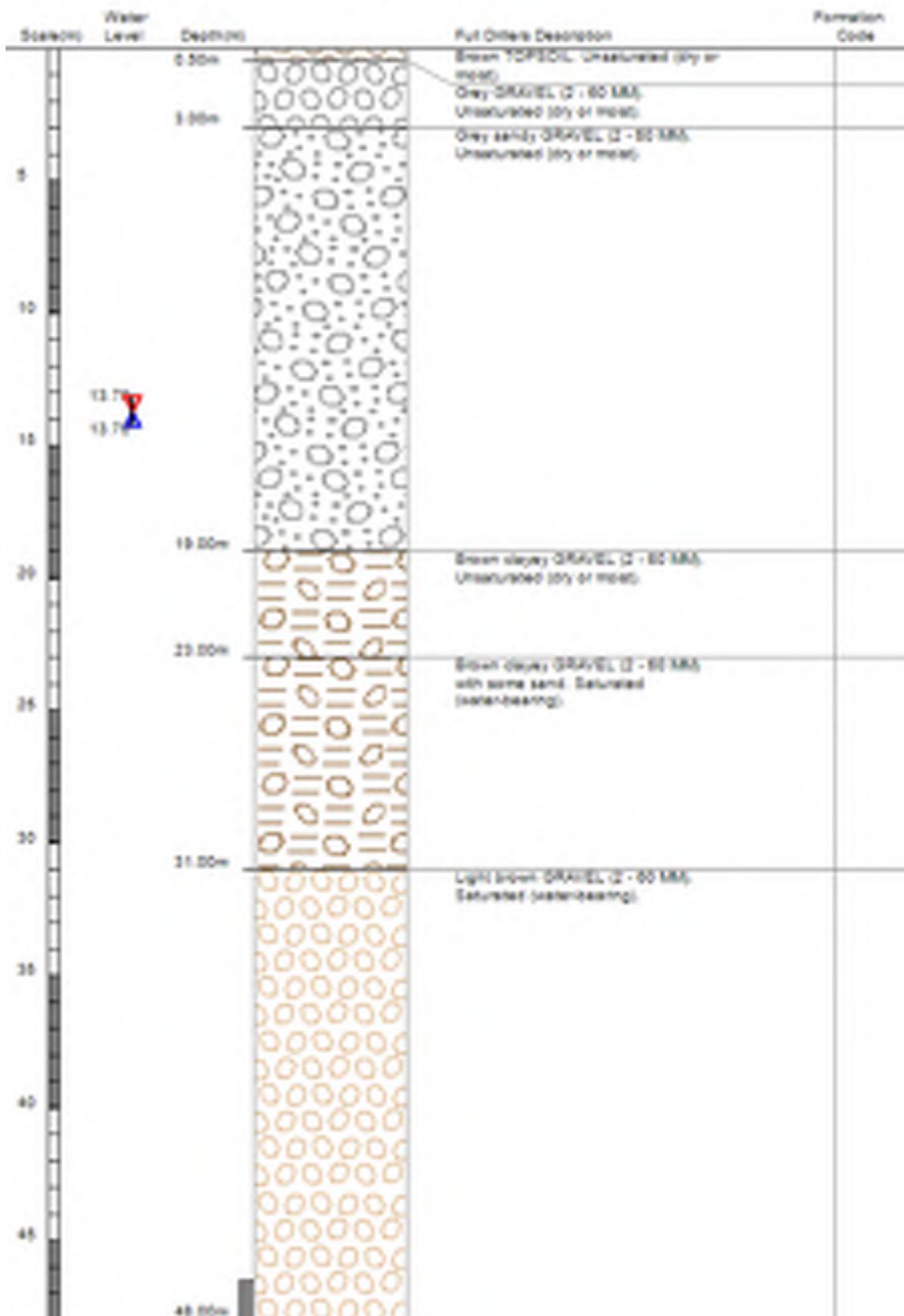
Borelog for well M36/4966

Grid Reference (NZTM): 1552787 mE, 5171568 mN
 Location Accuracy: 50 - 200m
 Ground Level Altitude: 38.8 m +MSD Accuracy: < 2.5 m
 Driller: McMillan Drilling Ltd
 Drill Method: Rotary/Percussion
 Borelog Depth: 48.5 m Drill Date: 18-Aug-1995



Borelog for well BX23/0533

Grid Reference (NZTM): 1552674 mE, 5171682 mN
 Location Accuracy: 10 - 50m
 Ground Level Altitude: m +MSD Accuracy:
 Driller: East Coast Drilling
 Drill Method: Air Rotary
 Borelog Depth: 48.0 m Drill Date: 20-Nov-2015



PROJECT **Branthwaite Drive**

 METHOD **TP**

CO-ORDINATES (NZTM)

LOGGED

CHECKED

 MACHINE & NO. **Wheeled Excavator**
E 1552186
T. MITCHELL
A. HILLS
N 5171475

 CONTRACTOR **Maugers**

 GROUND LEVEL **+37.00** m RL

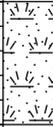
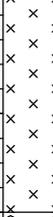
DATE

DATE

22/11/2016
2/12/2016

STRATA

SAMPLES & TESTS

Depth (m)	Legend	Description	Depth	No	Remarks/Tests
0.50		SILT with minor sand and trace of rootlets; dark brown. Moist, low plasticity; sand, fine. (TOPSOIL)			
1.40		SILT; light brown with orange-grey mottles. Moist, low plasticity. 1.20 - 1.40 Becomes with minor sand.			
1.70		Fine to coarse GRAVEL with minor sand, silt and cobbles; brownish grey. Moist, subrounded to rounded; sand, fine to medium.			
		End of Trial pit/trench at 1.70m, on 22/11/2016 <i>Termination Reason:</i> Target depth acheived.			

GENERAL REMARKS

 SHORING/SUPPORT: **None**
 STABILITY: **Generally Stable**
Groundwater not encountered.
Coordinates found using handheld GPS, likely accurate to +/- 5 m.
Ground level found using handheld GPS, likely accurate to +/- 10 m.

All dimensions in metres

 CLIENT **GW Rolleston Ltd.**
 Pocket Penetrometer Test
 Insitu Vane Shear Test

 Water Level

PROJECT **Branthwaite Drive**

METHOD **TP**

CO-ORDINATES (NZTM)

LOGGED

CHECKED

MACHINE & NO. **Wheeled Excavator**

E 1552053

T. MITCHELL

A. HILLS

N 5171529

CONTRACTOR **Maugers**

GROUND LEVEL **+43.00** m RL

DATE

DATE

22/11/2016

2/12/2016

STRATA

SAMPLES & TESTS

Depth (m)	Legend	Description	Depth	No	Remarks/Tests
0.25		SILT with minor sand and some rootlets; dark brown. Moist, low plasticity; sand, fine. (TOPSOIL)			
0.70		SILT with minor sand; light brown. Moist, low plasticity; sand, fine.			
1.60		Fine to coarse GRAVEL with some sand; greyish brown. Moist, subrounded to rounded; sand, fine to coarse.			
End of Trial pit/trench at 1.60m, on 22/11/2016 <i>Termination Reason:</i> Target depth achieved.					

Report ID: AGS4 TEST PIT RECORD (NO SKETCH NO MAP) || Project: BRANTHWAITE DRIVE LOGS.GPJ || Library: AGS 4_0.GLB || Date: 5 December 2016

GENERAL REMARKS

SHORING/SUPPORT: **None**
STABILITY: **Generally Stable**

Groundwater not encountered.
Coordinates found using handheld GPS, likely accurate to +/- 5 m.
Ground level found using handheld GPS, likely accurate to +/- 10 m.

All dimensions in metres

CLIENT **GW Rolleston Ltd.**

 Pocket Penetrometer Test
 Insitu Vane Shear Test

 Water Level

PROJECT **Branthwaite Drive**

 METHOD **TP**

CO-ORDINATES (NZTM)

LOGGED

CHECKED

 MACHINE & NO. **Wheeled Excavator**
E 1552359
T. MITCHELL
A. HILLS

 CONTRACTOR **Maugers**
N 5171660

DATE

DATE

 GROUND LEVEL **+43.00** m RL

23/11/2016
5/12/2016

STRATA

SAMPLES & TESTS

Depth (m)	Legend	Description	Depth	No	Remarks/Tests
0.25		SILT with minor sand and rootlets; dark brown. Moist, low plasticity; sand, fine. (TOPSOIL)			
0.50		Silty fine SAND with trace of rootlets; brown. Dry.			
1.00		Fine to coarse GRAVEL with some sand, minor cobbles, trace of rootlets and occasional boulders; brown. Dry, subrounded to rounded; sand, fine to coarse.			
1.60		1.00 Becomes with no rootlets; greyish brown.			
		End of Trial pit/trench at 1.60m, on 23/11/2016 <i>Termination Reason:</i> Target depth acheived.			

GENERAL REMARKS

 SHORING/SUPPORT: **None**
 STABILITY: **Generally Stable**
Groundwater not encountered.
Coordinates found using handheld GPS, likely accurate to +/- 5 m.
Ground level found using handheld GPS, likely accurate to +/- 10 m.

All dimensions in metres

 CLIENT **GW Rolleston Ltd.**
 Pocket Penetrometer Test
 Insitu Vane Shear Test

 Water Level

PROJECT **Branthwaite Drive**

 METHOD **TP**

CO-ORDINATES (NZTM)

LOGGED

CHECKED

 MACHINE & NO. **Wheeled Excavator**
E 1552208
T. MITCHELL
A. HILLS

 CONTRACTOR **Maugers**
N 5171608

DATE

DATE

 GROUND LEVEL **+44.00** m RL

23/11/2016
5/12/2016

STRATA

SAMPLES & TESTS

Depth (m)	Legend	Description	Depth	No	Remarks/Tests
0.20		SILT with minor sand and tree roots (up to 10 mm); dark brown. Moist, low plasticity; sand, fine. (TOPSOIL)			
0.80		SILT with minor sand; brown. Moist, low plasticity; sand, fine.			
1.70		Fine to coarse GRAVEL with some sand, minor cobbles and trace of rootlets; light brown. Moist, subrounded to rounded; sand, medium. 1.20 Becomes with no rootlets.			
		End of Trial pit/trench at 1.70m, on 23/11/2016 <i>Termination Reason:</i> Target depth acheived.			

GENERAL REMARKS

 SHORING/SUPPORT: **None**
 STABILITY: **Generally Stable**
Groundwater not encountered.
Coordinates found using handheld GPS, likely accurate to +/- 5 m.
Ground level found using handheld GPS, likely accurate to +/- 10 m.

All dimensions in metres

 CLIENT **GW Rolleston Ltd.**
 Pocket Penetrometer Test
 Insitu Vane Shear Test

 Water Level

PROJECT **Branthwaite Drive**

 METHOD **TP**

CO-ORDINATES (NZTM)

LOGGED

CHECKED

 MACHINE & NO. **Wheeled Excavator**
E 1552490
T. MITCHELL
A. HILLS

 CONTRACTOR **Maugers**
N 5171658

DATE

DATE

 GROUND LEVEL **+44.00** m RL

23/11/2016
5/12/2016

STRATA

SAMPLES & TESTS

Depth (m)	Legend	Description	Depth	No	Remarks/Tests
0.35		SILT with minor sand and rootlets; dark brown. Moist, low plasticity; sand, fine. (TOPSOIL)			
0.70		SILT with some sand; brown. Moist, low plasticity; sand, fine.			
1.60		Fine to coarse GRAVEL with some sand and trace of rootlets; brownish grey. Moist, subrounded to rounded; sand, fine to medium. 0.70 - 0.80 Sand becomes medium to coarse, light brown. 1.30 Becomes with no rootlets.			
End of Trial pit/trench at 1.60m, on 23/11/2016 <i>Termination Reason:</i> Target depth achieved.					

GENERAL REMARKS

 SHORING/SUPPORT: **None**
 STABILITY: **Generally Stable**
Groundwater not encountered.
Coordinates found using handheld GPS, likely accurate to +/- 5 m.
Ground level found using handheld GPS, likely accurate to +/- 10 m.

All dimensions in metres

 CLIENT **GW Rolleston Ltd.**
 Pocket Penetrometer Test
 Insitu Vane Shear Test

 Water Level

		Client: Hank Developments Limited Project: Proposed Subdivision Address: 7/572 Selwyn Road, Rolleston	Augerhole No. HA02 Sheet No. 1 of 1
Drill Type: 8 Ton Excavator Drilled By: BE Date Started: 6-Apr-18 Date Finished: 6-Apr-18	Project No: LTCL18051 Coordinates: NZTM: 1552207 mE, 5171344 mN Ground Conditions: Grassed, Near level Groundwater Level (m): Not Encountered (6-Apr-18)	Logged By: BE Shear Vane No: N/A Calibration Factor: N/A Calibration Date: N/A	
Stratigraphy Depth (m) Graphic Log	Soil description in accordance with <i>Guideline for the Field Classification and Description of Soil and Rock for Engineering Purposes</i> , NZ Geotechnical Society Inc., 2005	Groundwater Level (m) Depth (m)	In-situ Field Testing Shear Strength (kPa) Dynamic Cone Penetrometer Peak: ● Remoulded: ● Depth (m) Blow Count Scala Blow Count / 100mm 0 5 10 15 20
TOPSOIL 0.5 RIVER DEPOSITS 1.0 1.5 2.0 2.5 3.0 3.5 4.0 4.5 5.0	SILT, minor fine sand, minor organics, dark brown, medium dense, moist, non-plastic [TOPSOIL] SILT, minor fine sand, trace subrounded greywacke gravel, yellowish brown, dense, moist, non-plastic [RIVER DEPOSITS] Fine to coarse sandy fine to coarse subrounded greywacke GRAVEL, trace to minor subrounded greywacke cobbles, greyish brown, tightly packed, moist End of Test Pit (2.2m)	0 0.5 1.0 1.5 2.0 2.5 3.0 3.5 4.0 4.5 5.0	-0.1 3 -0.2 3 -0.3 4 -0.4 7 -0.5 10 -0.6 11 -0.7 12 -0.8 10 -0.9 19 -1.0 25 + -1.1 -1.2 -1.3 -1.4 -1.5 -1.6 -1.7 -1.8 -1.9 -2.0 -2.1 -2.2 -2.3 -2.4 -2.5 -2.6 -2.7 -2.8 -2.9 -3.0 -3.1 -3.2 -3.3 -3.4 -3.5 -3.6 -3.7 -3.8 -3.9 -4.0 -4.1 -4.2 -4.3 -4.4 -4.5 -4.6 -4.7 -4.8 -4.9 -5.0
In-situ field testing in accordance with the following Standards: Scala Penetrometer Testing: NZS 4402:1988, Test 6.5.2, Dynamic Cone Penetrometer Shear Vane Testing: Guideline for Hand Held Shear Vane Test, NZGS, August 2001			

		Client: Hank Developments Limited Project: Proposed Subdivision Address: 7/572 Selwyn Road, Rolleston	Augerhole No. HA03 Sheet No. 1 of 1
Drill Type: 8 Ton Excavator Drilled By: BE Date Started: 6-Apr-18 Date Finished: 6-Apr-18	Project No: LTCL18051 Coordinates: NZTM: 1552231 mE, 5171302 mN Ground Conditions: Grassed, Near level Groundwater Level (m): Not Encountered (6-Apr-18)	Logged By: BE Shear Vane No: N/A Calibration Factor: N/A Calibration Date: N/A	
Stratigraphy Depth (m) Graphic Log	Soil description in accordance with <i>Guideline for the Field Classification and Description of Soil and Rock for Engineering Purposes</i> , NZ Geotechnical Society Inc., 2005	Groundwater Level (m) Depth (m)	In-situ Field Testing Shear Strength (kPa) Dynamic Cone Penetrometer Peak: —●— Remoulded: ● Depth (m) Blow Count Scala Blow Count / 100mm 0 5 10 15 20
TOPSOIL 0.5 RIVER DEPOSITS 1.0 1.5 2.0 2.5 3.0 3.5 4.0 4.5 5.0	SILT, minor fine sand, minor organics, dark brown, medium dense, moist, non-plastic [TOPSOIL] SILT, minor fine sand, yellowish brown, dense, moist, non-plastic [RIVER DEPOSITS] Fine to coarse sandy, fine to coarse subrounded greywacke GRAVEL, trace subrounded cobbles, greyish brown, tightly packed, moist End of Test Pit (2.3m)	0.5 1.0 1.5 2.0 2.5 3.0 3.5 4.0 4.5 5.0	-0.1 3 -0.2 2 -0.3 4 -0.4 6 -0.5 10 -0.6 12 -0.7 25 + -0.8 -0.9 -1.0 -1.1 -1.2 -1.3 -1.4 -1.5 -1.6 -1.7 -1.8 -1.9 -2.0 -2.1 -2.2 -2.3 -2.4 -2.5 -2.6 -2.7 -2.8 -2.9 -3.0 -3.1 -3.2 -3.3 -3.4 -3.5 -3.6 -3.7 -3.8 -3.9 -4.0 -4.1 -4.2 -4.3 -4.4 -4.5 -4.6 -4.7 -4.8 -4.9 -5.0
			In-situ field testing in accordance with the following Standards: Scala Penetrometer Testing: NZS 4402:1988, Test 6.5.2, Dynamic Cone Penetrometer Shear Vane Testing: Guideline for Hand Held Shear Vane Test, NZGS, August 2001

		Client: Hank Developments Limited Project: Proposed Subdivision Address: 7/572 Selwyn Road, Rolleston	Augerhole No. HA04 Sheet No. 1 of 1
Drill Type: 8 Ton Excavator Drilled By: BE Date Started: 6-Apr-18 Date Finished: 6-Apr-18	Project No: LTCL18051 Coordinates: NZTM: 1552136 mE, 5171389 mN Ground Conditions: Grassed, Near level Groundwater Level (m): Not Encountered (6-Apr-18)	Logged By: BE Shear Vane No: N/A Calibration Factor: N/A Calibration Date: N/A	
Stratigraphy Depth (m) Graphic Log	Soil description in accordance with <i>Guideline for the Field Classification and Description of Soil and Rock for Engineering Purposes</i> , NZ Geotechnical Society Inc., 2005	Groundwater Level (m) Depth (m)	In-situ Field Testing Shear Strength (kPa) Dynamic Cone Penetrometer Peak: ● Remoulded: ● Depth (m) Blow Count Scala Blow Count / 100mm 0 5 10 15 20
TOPSOIL 0.5 RIVER DEPOSITS 1.0 1.5 2.0 2.5 3.0 3.5 4.0 4.5 5.0	SILT, minor fine sand, minor organics, dark brown, medium dense, moist, non-plastic [TOPSOIL] SILT, minor fine sand, yellowish brown, dense, moist, non-plastic [RIVER DEPOSITS] Fine to coarse sandy fine to coarse subrounded greywacke GRAVEL, trace to minor subrounded greywacke cobbles, greyish brown, lightly packed, moist End of Test Pit (2.2m)	0.5 1.0 1.5 2.0 2.5 3.0 3.5 4.0 4.5 5.0	-0.1 3 -0.2 4 -0.3 3 -0.4 8 -0.5 10 -0.6 12 -0.7 25 + -0.8 -0.9 -1.0 -1.1 -1.2 -1.3 -1.4 -1.5 -1.6 -1.7 -1.8 -1.9 -2.0 -2.1 -2.2 -2.3 -2.4 -2.5 -2.6 -2.7 -2.8 -2.9 -3.0 -3.1 -3.2 -3.3 -3.4 -3.5 -3.6 -3.7 -3.8 -3.9 -4.0 -4.1 -4.2 -4.3 -4.4 -4.5 -4.6 -4.7 -4.8 -4.9 -5.0
			In-situ field testing in accordance with the following Standards: Scala Penetrometer Testing: NZS 4402:1988, Test 6.5.2, Dynamic Cone Penetrometer Shear Vane Testing: Guideline for Hand Held Shear Vane Test, NZGS, August 2001

		Client: Hank Developments Limited Project: Proposed Subdivision Address: 7/572 Selwyn Road, Rolleston	Augerhole No. HA07 Sheet No. 1 of 1
Drill Type: 8 Ton Excavator Drilled By: BE Date Started: 6-Apr-18 Date Finished: 6-Apr-18	Project No: LTCL18051 Coordinates: NZTM: 1552139 mE, 5171345 mN Ground Conditions: Grassed, Near level Groundwater Level (m): Not Encountered (6-Apr-18)	Logged By: BE Shear Vane No: N/A Calibration Factor: N/A Calibration Date: N/A	
Stratigraphy Depth (m) Graphic Log	Soil description in accordance with <i>Guideline for the Field Classification and Description of Soil and Rock for Engineering Purposes</i> , NZ Geotechnical Society Inc., 2005	Groundwater Level (m) Depth (m)	In-situ Field Testing Shear Strength (kPa) Peak: ● Remoulded: ● Dynamic Cone Penetrometer Depth (m) Blow Count Scala Blow Count / 100mm 0 5 10 15 20
TOPSOIL 0.5 RIVER DEPOSITS 1.0 1.5 2.0 2.5 3.0 3.5 4.0 4.5 5.0	SILT, minor fine sand, minor organics, dark brown, medium dense, moist, non-plastic [TOPSOIL] SILT, minor fine sand, yellowish brown, dense, moist, non-plastic [RIVER DEPOSITS] Fine to coarse sandy fine to coarse subrounded greywacke gravel, greyish brown, tightly packed, moist [RIVER DEPOSITS] trace to minor subrounded greywacke cobbles End of Test Pit (2.6m)	0 0.5 1.0 1.5 2.0 2.5 3.0 3.5 4.0 4.5 5.0	-0.1 3 -0.2 3 -0.3 4 -0.4 6 -0.5 8 -0.6 11 -0.7 25 + -0.8 -0.9 -1.0 -1.1 -1.2 -1.3 -1.4 -1.5 -1.6 -1.7 -1.8 -1.9 -2.0 -2.1 -2.2 -2.3 -2.4 -2.5 -2.6 -2.7 -2.8 -2.9 -3.0 -3.1 -3.2 -3.3 -3.4 -3.5 -3.6 -3.7 -3.8 -3.9 -4.0 -4.1 -4.2 -4.3 -4.4 -4.5 -4.6 -4.7 -4.8 -4.9 -5.0
			In-situ field testing in accordance with the following Standards: Scala Penetrometer Testing: NZS 4402:1988, Test 6.5.2, Dynamic Cone Penetrometer Shear Vane Testing: Guideline for Hand Held Shear Vane Test, NZGS, August 2001

		Client: Hank Developments Limited Project: Proposed Subdivision Address: 7/572 Selwyn Road, Rolleston	Augerhole No. HA05 Sheet No. 1 of 1
Drill Type: 8 Ton Excavator Drilled By: BE Date Started: 6-Apr-18 Date Finished: 6-Apr-18	Project No: LTCL18051 Coordinates: NZTM: 1552187 mE, 5171307 mN Ground Conditions: Grassed, Near level Groundwater Level (m): Not Encountered (6-Apr-18)	Logged By: BE Shear Vane No: N/A Calibration Factor: N/A Calibration Date: N/A	
Stratigraphy Depth (m) Graphic Log	Soil description in accordance with <i>Guideline for the Field Classification and Description of Soil and Rock for Engineering Purposes</i> , NZ Geotechnical Society Inc., 2005	Groundwater Level (m) Depth (m)	In-situ Field Testing Shear Strength (kPa) Peak: ● Remoulded: ● Dynamic Cone Penetrometer Depth (m) Blow Count Scala Blow Count / 100mm 0 5 10 15 20
TOPSOIL 0.5 RIVER DEPOSITS 1.0 1.5 2.0 2.5 3.0 3.5 4.0 4.5 5.0	SILT, minor fine sand, minor organics, dark brown, loose, moist, non-plastic [TOPSOIL] SILT, minor fine sand, yellowish brown, medium dense, moist, non-plastic [RIVER DEPOSITS] Fine to coarse sandy fine to coarse subrounded greywacke GRAVEL, trace subrounded greywacke cobbles, greyish brown, tightly packed, moist End of Test Pit (2.1m)	0 0.5 1.0 1.5 2.0 2.5 3.0 3.5 4.0 4.5 5.0	-0.1 -0.2 -0.3 -0.4 -0.5 -0.6 -0.7 -0.8 -0.8 -0.9 -1.0 -1.1 -1.2 -1.3 -1.4 -1.5 -1.6 -1.7 -1.8 -1.9 -2.0 -2.1 -2.2 -2.3 -2.4 -2.5 -2.6 -2.7 -2.8 -2.9 -3.0 -3.1 -3.2 -3.3 -3.4 -3.5 -3.6 -3.7 -3.8 -3.9 -4.0 -4.1 -4.2 -4.3 -4.4 -4.5 -4.6 -4.7 -4.8 -4.9 -5.0
			In-situ field testing in accordance with the following Standards: Scala Penetrometer Testing: NZS 4402:1988, Test 6.5.2, Dynamic Cone Penetrometer Shear Vane Testing: Guideline for Hand Held Shear Vane Test, NZGS, August 2001

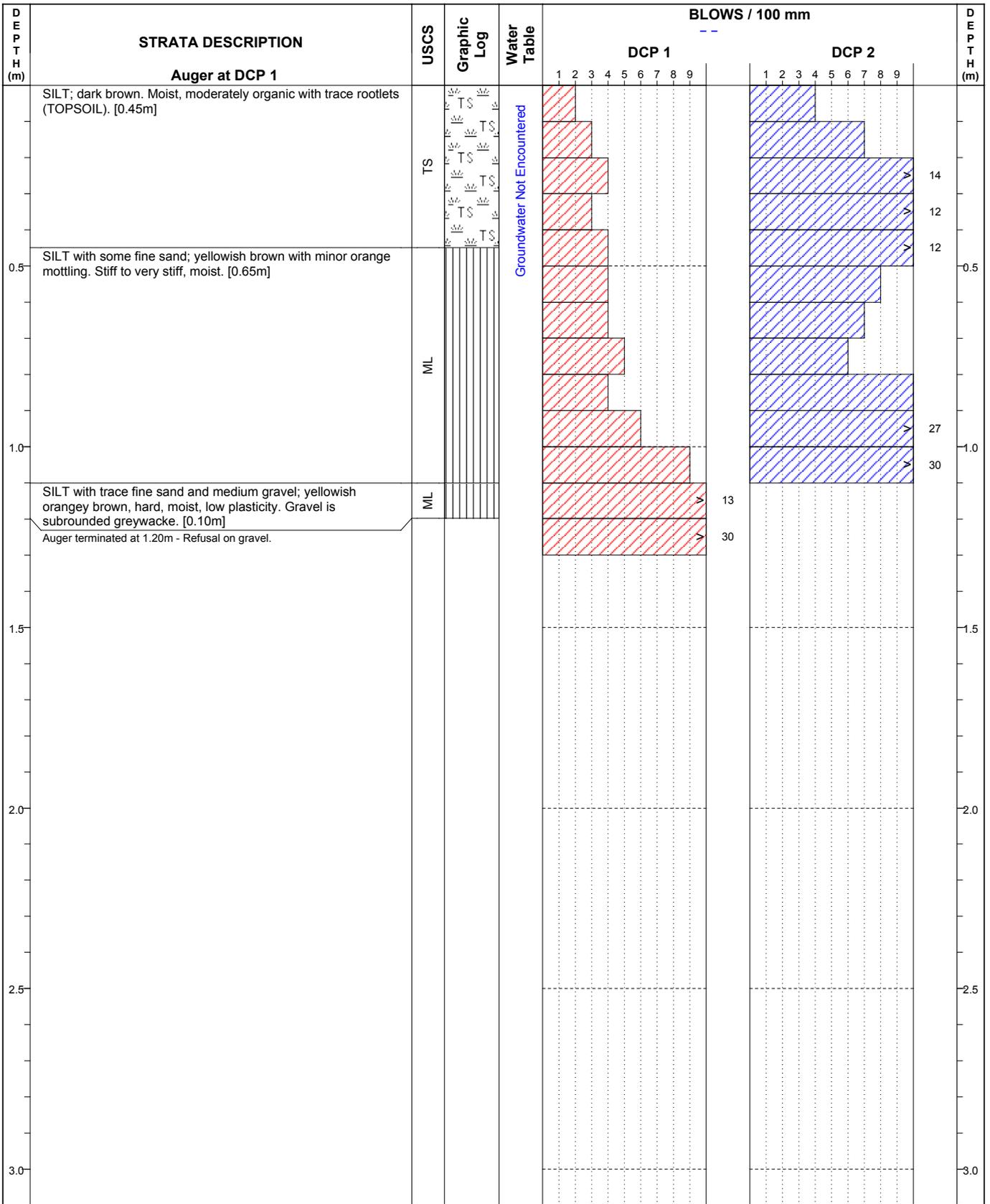
		Client: Hank Developments Limited Project: Proposed Subdivision Address: 7/572 Selwyn Road, Rolleston	Augerhole No. HA06 Sheet No. 1 of 1
Drill Type: 8 Ton Excavator Drilled By: BE Date Started: 6-Apr-18 Date Finished: 6-Apr-18	Project No: LTCL18051 Coordinates: NZTM: 1552211 mE, 5171252 mN Ground Conditions: Grassed, Near level Groundwater Level (m): Not Encountered (6-Apr-18)	Logged By: BE Shear Vane No: N/A Calibration Factor: N/A Calibration Date: N/A	
Stratigraphy Depth (m) Graphic Log	Soil description in accordance with <i>Guideline for the Field Classification and Description of Soil and Rock for Engineering Purposes</i> , NZ Geotechnical Society Inc., 2005	Groundwater Level (m) Depth (m)	In-situ Field Testing Shear Strength (kPa) Peak: —●— Remoulded: ● Dynamic Cone Penetrometer Depth (m) Blow Count Scala Blow Count / 100mm 0 5 10 15 20
TOPSOIL 0.5 RIVER DEPOSITS 1.0 1.5 2.0 2.5 3.0 3.5 4.0 4.5 5.0	SILT, minor fine sand, minor organics dark brown, loose, moist, non-plastic [TOPSOIL] SILT, minor to some fine sand, yellowish brown, medium dense, moist, non-plastic [RIVER DEPOSITS] Fine to coarse sandy fine to coarse subrounded greywacke GRAVEL, trace subrounded greywacke cobbles, greyish brown, tightly packed, moist End of Test Pit (2.2m)	0 0.5 1.0 1.5 2.0 2.5 3.0 3.5 4.0 4.5 5.0	-0.1 2 -0.2 3 -0.3 3 -0.4 4 -0.5 5 -0.6 5 -0.7 25 + -0.8 -0.9 -1.0 -1.1 -1.2 -1.3 -1.4 -1.5 -1.6 -1.7 -1.8 -1.9 -2.0 -2.1 -2.2 -2.3 -2.4 -2.5 -2.6 -2.7 -2.8 -2.9 -3.0 -3.1 -3.2 -3.3 -3.4 -3.5 -3.6 -3.7 -3.8 -3.9 -4.0 -4.1 -4.2 -4.3 -4.4 -4.5 -4.6 -4.7 -4.8 -4.9 -5.0
In-situ field testing in accordance with the following Standards: Scala Penetrometer Testing: NZS 4402:1988, Test 6.5.2, Dynamic Cone Penetrometer Shear Vane Testing: Guideline for Hand Held Shear Vane Test, NZGS, August 2001			

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 www.do.nz

SHALLOW INVESTIGATION RESULTS

Job N^o /39353
 Test N^o /DCP 1 + HA DCP 2

Project: 19 Raptor Street, Falcons Landing, Rolleston (Lot 298 DP 532807)	Date: 28/08/19
Client: Compass Homes	Time: 10:00 a.m.
Test Location: Refer to attached Geotechnical Site Plan (DWG G01A)	Excavation Method: DCP+HA



Produced with Core-GS by Geroc

Logged By: HC+GC
Plotted By: GC
Checked By: HC

Notes:

Dynamic Penetrometer Test and logs give an indication of the ground condition at the location of the tests only. While they are representative of typical conditions across the site, they do not identify variations in the ground away from the test locations. This log does not cover slope stability or suitability of the site for building.

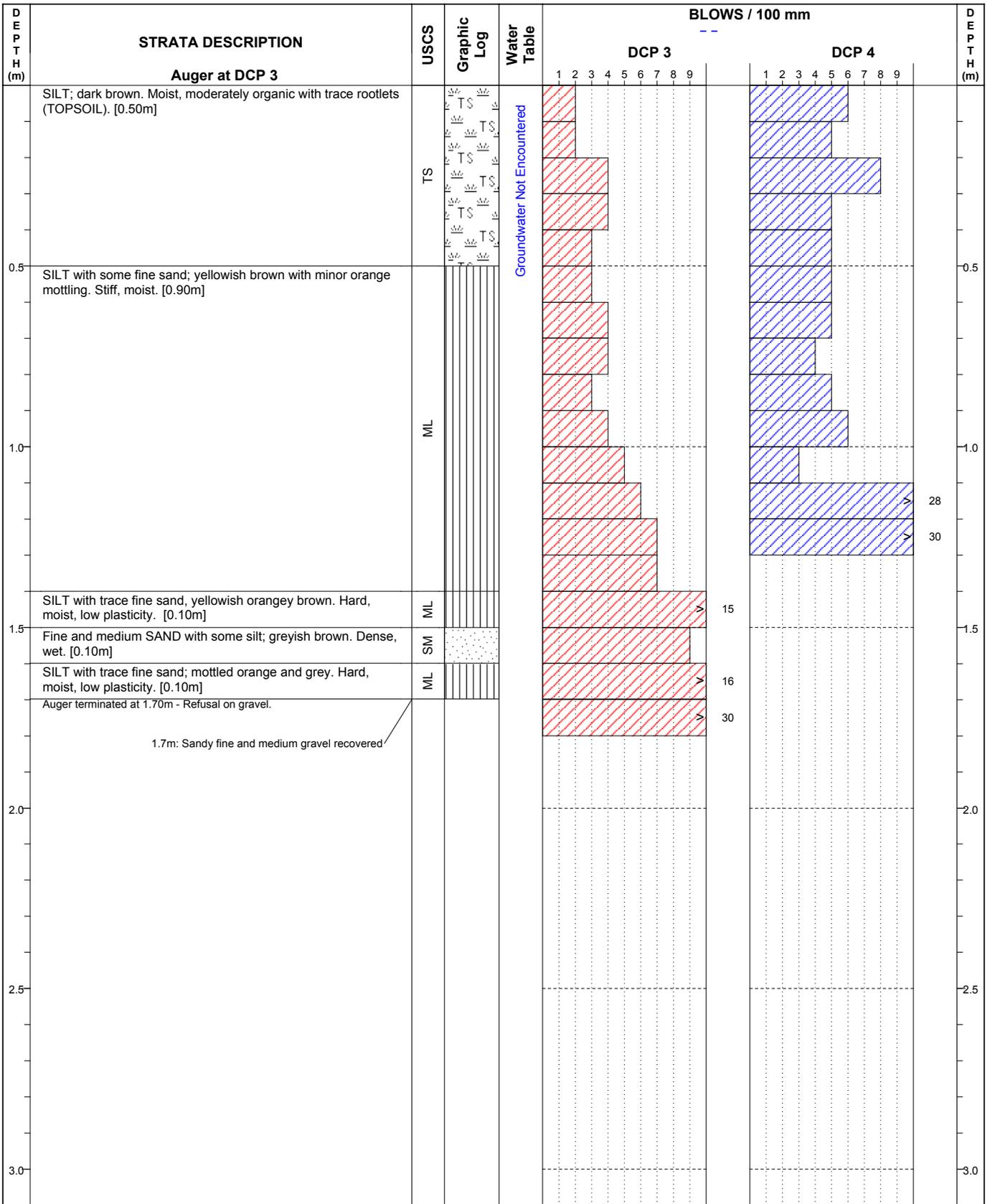
Dynamic Cone Penetrometer Test performed in accordance with NZS 4402 Test 6.5.2 (Procedure 1 and 2)



SHALLOW INVESTIGATION RESULTS

Job N^o /39353
 Test N^o /DCP 3 + HA DCP 4

Project: 19 Raptor Street, Falcons Landing, Rolleston (Lot 298 DP 532807)	Date: 28/08/19
Client: Compass Homes	Time: 10:00 a.m.
Test Location: Refer to attached Geotechnical Site Plan (DWG G01A)	Excavation Method: DCP+HA



Produced with Core-GS by Geroc

Logged By: HC+GC	Notes: Dynamic Penetrometer Test and logs give an indication of the ground condition at the location of the tests only. While they are representative of typical conditions across the site, they do not identify variations in the ground away from the test locations. This log does not cover slope stability or suitability of the site for building. Dynamic Cone Penetrometer Test performed in accordance with NZS 4402 Test 6.5.2 (Procedure 1 and 2)
Plotted By: GC	
Checked By: HC	